# **TRAFFIC IMPACT STUDY**

# SWAMPSCOTT MIDDLE and ELEMENTARY SCHOOLS



Prepared for:

Mount Vernon Group Architects 200 Harvard Square, Suite 410 Wakefield, MA 01800

Prepared by:

GEOD Consulting, Inc. 24 Ray Avenue Burlington, MA 01803 Phone No.: 781-273-3434



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### INTRODUCTION

The Swampscott School District is proposing to develop a portion of the Middle School site by adding an Elementary School which will encompass 635 students. The site is located at 207 Forest Avenue, Swampscott MA. The Middle School presently has 602 students and will continue to maintain this number after the Elementary School is constructed. The Middle School starts at 8:00am and the last regular class gets out at 2:20pm. The purpose of this Traffic Impact Study (TIS) Report is to assess the existing traffic conditions and the impacts of the Elementary School on the area street network. The site is located on the north side of Forest Avenue and is bounded by Forest Ave., Laurel Rd. to the east, playfields to the north, and Burke Dr. to the west. The proposed Elementary School is expected to be constructed to the northeast, or behind the Middle School on existing playfields with a direct connection to Nason Rd and a driveways to Forest Ave.

A locus map, **Figure 1**, Location Plan is provided with the site centrally located on a USGS map. A Site Plan, **Figure 2**, indicating the proposed Elementary School footprint relative to existing site conditions, including the Middle School and local street network is provided by the project architect, Mount Vernon Group (MVG).

GEOD Consulting Inc. has evaluated the existing traffic condition and the impacts of the new Elementary School. The results are presented in this Study Report. The Report includes an analysis of the existing study area intersections and roadways, and the future condition with the inclusion of the Elementary School.

### ✤ PROPOSED ELEMENTARY SCHOOL

The proposed Elementary School (ES) will include 635 students, teachers and administration staff necessary to operate the school. The students are presently located at three schools in Town, Hadley ES on Redington St, Clarke ES on Middlesex Ave. and Stanley ES on Whitman Rd. The elementary students at these three schools will be relocated to the new school location. The Hadley, Clarke and Stanley ES's are located southwest, northwest and east of the new ES, respectively. The ES hours are from 8:20am to 2:15pm.

The proposed ES will be accessed by three avenues, a continuation of Nason Rd, the existing eastern driveway and the main MS driveway both intersecting with Forest Avenue.

### PROJECT STUDY AREA

GEOD Consulting has met with the school building committee, the project team and town personnel on several occasions. We have visited the site during the peak AM and PM periods and assessed the existing conditions. The project study area, affected by the existing MS and the future ES, will be Forest Avenue, Laurel Road, Sargent Road and Nason Road. Each of these either presently or in the future will be the primary access routes to and from the schools. The schools will be directly accessible by Forest Ave. and Nason Rd. All of the roadways are owned and maintained by the Town of Swampscott.

All of the study driveways and intersection are unsignalized. The following intersection/driveways comprise the study area:



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- Forest Avenue/MS western driveway, exit only.
- Forest Avenue/MS central driveway, entrance only.
- Forest Avenue/MS eastern driveway, exit and entrance.
- Forest Avenue/Sargent Road.
- Nason Road driveway to the new ES.

### **EXISTING CONDITIONS**

Field investigations were conducted at the study area roadways and intersection/driveways to obtain an inventory of existing conditions, posted traffic controls, adjacent land uses, lane configurations, pavement widths, and existing vehicular traffic patterns.

### ROADWAYS

### **♦** FOREST AVENUE

Forest Ave. is a two-lane east-west local roadway under the Town's jurisdiction, which ultimately connects Humphrey St. (Rte 129) to Paradise Rd. (Rte 1A). The roadway is relatively flat in the study area with a posted speed limit of 20 mph. The area is exclusively residential. The pavement condition on Forest Ave. is fair with minor repairs and cracks and the roadway width is approximately 33 feet. The length of Forest Ave. in front of the MS has a sidewalk on the northerly side adjacent to the school, but not on the southerly side. Crosswalks are provided at the main driveway entrance and at the intersection with Sargent Rd. The north side of the roadway, adjacent to the MS, is signed for "1 Hr Parking, 8am - 3pm", and the south side of the road is signed for "No Parking, 8am - 3pm".

### SARGENT ROAD

Sargent Rd. is a two-lane north-south local roadway under the Town's jurisdiction. The roadway connects Forest Avenue to Pleasant Street, and then to Humphrey St. (Rte 129). The roadway has significant grades, however it's relatively flat before it intersects with Forest Ave. The roadway does not have a posted speed limit, however we assume it to be 20 mph. The area is exclusively residential. The pavement condition on Sargent Road Ave. is fair to poor with numerous pavement deficiencies. The roadway width is approximately 34 feet between Forest Ave. and Pleasant Street, and beyond Pleasant St. the width decreases to varying widths.

### ✤ NASON ROAD

Nason Rd. is a two-lane east-west local roadway under the Town's jurisdiction, which connects Humphrey St. (Rte 129) to Laurel Rd. and then onto Forest Ave. The roadway has moderate grades in the study area with a posted speed limit of 20 mph. The area is exclusively residential. The pavement condition on Nason Rd. is fair with minor repairs and cracks, and the roadway width is approximately 32 feet. Nason Rd. has sidewalks on both sides of the road and is signed for no parking.

### ✤ LAUREL ROAD

Laurel Rd. is a two-lane north-south local roadway under the Town's jurisdiction, which connects Nason Rd to Forest Ave. The roadway has minimal grades in the study area. The roadway does not have a posted speed limit, however we assume it to be 20 mph. The area is exclusively residential.

The pavement condition on Laurel Rd. is fair with minor repairs and cracks and the roadway width is approximately 23 feet. Laurel Rd. has sidewalks on both sides of the road and is signed for no parking.

### TRAFFIC COUNT VOLUMES

Existing traffic counts were obtained from Transportation Data Corporation (TDC) for the study area intersection and driveways. The actual count data obtained by TDC is shown in **Appendix A**. Turning movement counts were collected on Tuesday, April 1, 2014 from 7:00 am to 9:00 am and from 2:00 pm to 4:00 pm. Additional counts were performed by Marshall-Gary Landscape Architects a couple of months previous to the TDC counts.

The existing turning movement counts for the AM and PM peak hours are shown in Figure 3.

### SAFETY ANALYSIS

The Swampscott Police Department has been contacted to request accident traffic history for the project study area over the last three years. We have not received a report, however we were told verbally that there is not a history of accidents in the study area.

#### **OPERATIONAL ANALYSIS**

The Highway Capacity Manual (HCM) methodology uses LOS as an index to the operational qualities of an intersection. Level of service measures the operating conditions of signalized and unsignalized intersections for a specific period of time, which is usually the peak hour. The methodology evaluates operating conditions and assigns a level of service rating from LOS "A" to "F" with LOS A being the most desirable. The LOS is a function of the average vehicle delay at the intersection. **Table 1** illustrates the LOS thresholds for unsignalized and signalized intersections.

TABLE 1												
LEVEL of SERVICE (LOS) DELAY THRESHOLD for INTERSECTIONS												
UNSIGNALIZED INTERSECTION AVERAGE VEHICLE DELAY (SECONDS) SIGNALIZED INTERSECTION AVERAGE VEHICLE DELAY (SECONDS) LEVEL OF SERVICE EXPECTED DELAY												
Equal to or Less than 10.0	Equal to or Less than 10.0	А	Little or no delay									
Between 10.1 and 15.0	Between 10.1 and 20.0	В	Short traffic delays									
Between 15.1 and 25.0	Between 20.1 and 35.0	С	Average traffic delays									
Between 25.1 and 35.0	Between 35.1 and 55.0	D	Long traffic delays									
Between 35.1 and 50.0 Between 55.1 and 80.0 E Very long traffic delays												
Greater than 50.0	Greater than 80.0	F	Forced flow									

The existing Middle School traffic operations were analyzed at the study area intersection and driveways, and are presented in **Table 2**. The LOS, volume to capacity ratios (v/c), and delay for the intersections existing condition have been calculated. The intersections operational/capacity analysis



calculations are shown in **Appendix B**. Typically the model calculations gives us an understanding of the operational capacity of the school site drives and the Forest Ave. / Sargent Road intersection. However, due to the excessive student drop offs on Forest Ave., and the crossing guards preference to student walkers at the expense of the vehicles in queue, the human element is driving the modeling software to determinations which do not match field observations. Consequently, our analysis is derived by a combination of modeling analysis and field observations.

### TRAFFIC ANALYSIS METHODOLOGY

The methodology used for the analysis and development of the TIS was a means to assess the existing operational condition of the study area roadways and intersections, add the ES generated traffic and additional access and egress points, ie. the Nason Rd and eastern driveway connections, and re-assess the proposed operational conditions. To do this TMC's were obtained as previously described, the existing conditions of the intersection/driveways were modeled and assessed, the proposed ES traffic and network then added to the model, and the study area reassessed. The intersection/driveways were analyzed and the operational conditions (level of service, intersection delay, approach delays, v/c capacity, queue lengths, etc.) calculated. The existing condition, MS and the future condition, with the ES, are then evaluated.

The existing, and proposed condition operational analyses were performed using SYNCRO 8.05; Traffic Control Coordination Software for the studied intersections. The software utilizes the methods for intersection analyses contained in the latest version of the Highway Capacity Manual (HCM), Special Report 209; Transportation Research Board.

This project is classified as educational with peaks in the AM (7 - 9am) and PM (2 - 4pm) time periods. Consequently, all conditions were analyzed for these two peak hours. The AM peak period will have significantly greater volume of traffic in a more condense time than the PM period because after school programs, for both the MS and ES will span out the PM peak period, thereby reducing the net effect to the study area network.

### **\*** TRIP GENERATION

In order to assess the volume of traffic generated by the three ES's (Stanley, Hadley and Clarke), traffic counts were taken at each of the schools. Each school has kindergarten students which will not be coming to the MS site. Consequently we reduced the trip counts by the percentage of kindergarten students in the schools.

Another consideration for the trip generation, and the volume of existing trips visiting the site is the number of families that have students in both the MS and ES. Presently, of the 602 students in the MS, 202 of them have siblings in the ES. Therefore 33% of the generated trips are already coming to the site. To be conservative we have reduced the trip generation by 20%, simply because we don't know if the 33% overlap is consistent year in and year out.

The following illustrates the ES volume of trips expected to come to the MS site:

Elementary School	Trips to School	% Kindergarten	ES Trips to School	20% Reduction	Adj. ES Trips
Clarke	101	17%	84	16	68
Hadley	94	21%	75	15	60
Stanley	144	21%	114	22	92
			TOTAL	ES TRIPS =	220

### AM Peak Hour

### PM Peak Hour

Elementary School	Trips to School	% Kindergarten	ES Trips to School	20% Reduction	Adj. ES Trips
Clarke	34	17%	29	6	23
Hadley	70	21%	56	12	44
Stanley	105	21%	83	17	66
			TOTAL	ES TRIPS =	133

TOTAL ES TRIPS = 

### **\*** TRIP DISTRIBUTION

The trip distribution is illustrated in Figure 4, Trip Distribution – AM and PM Peak Hour Volumes. Three routes provide access and egress to/from the site, Forest Ave. from the west, Sargent Rd. from the south and Nason Rd/Laurel Rd/Forest Ave from the east. The distribution from each route was developed utilizing a combination of methods. First we calculated the existing distribution based on the existing MS traffic pattern, and then we considered the location of the three existing ES's relative to the MS site. The existing traffic pattern illustrated that approximately 50% of the traffic comes to the MS from the west on Forest Ave, 20% from the south on Sargent Rd and 30% from the east on Nason Rd. The Clarke, Hadlley and Stanley schools are west, southwest and east of the MS, respectively. The three schools are similarly populated, and based on their locale to the MS, we believe the distribution will continue with the existing condition.

The trips generated by the ES are then distributed throughout the study area network utilizing the trip distribution. This is shown in **Figure 5**.

### **OPERATIONAL ANALYSIS – PROPOSED CONDITION**

At this point we have analyzed the existing roadway network condition as it operates with the Middle School. We then calculate the expected trips generated by the Elementary School, distribute the trips throughout the network, add those trips to the existing peak hour volumes as shown in Figure 3, and run a new model which will represent the Proposed Condition. The resulting proposed total volume coming to the site is shown in **Figure 6.** Lastly, we analyze the proposed network and if the network level of service (LOS) degenerates at any of the site drives to an unacceptable LOS we mitigate the condition.

Table 2 illustrates the operational capacity, delay and LOS, of the existing and proposed conditions at the site drives and Forest Ave/Sargent Rd intersection.







	TABLE 2														
DRIVEWAY / INTERSECTION - OPERATIONAL ANALYSIS															
DRIVEWAY/ INTERSECTION	MIDI AM	DLE SC PEAK I	HOOL HOUR	HOOL IOUR	DL MIDDLE & ELEM R PM PEAK HOUR										
DESCRIPTION	V/C <sup>1</sup>	<b>Delay</b> <sup>2</sup>	LOS <sup>3</sup>	V/C <sup>1</sup>	<b>Delay</b> <sup>2</sup>	LOS <sup>3</sup>	V/C <sup>1</sup>	Delay <sup>2</sup>	LOS <sup>3</sup>	V/C <sup>1</sup>	Delay <sup>2</sup>	LOS <sup>3</sup>			
Western Driveway	2	~	~	~	~	~	~	~	~	~	~	~			
Middle Driveway															
Driveway - Northbound	1.08	60.2	Е	1.53	264.5	F	0.23	3.6	А	0.87	24.90	С			
Forest Ave Westbound	0.93	35.2	D	~	~	~	0.47	6.7	А	~	~	~			
Forest Ave Eastbound	1.07	70.7	Е	~	~	~	0.62	8.9	А	~	~	~			
Eastern Driveway	2	~	~	~	~	~	~	~	~	~	~	~			
Forest Ave. / Sargent Rd.															
Sargent Rd Northbound	1.28	165.6	F	~	~	~	0.27	4.9	А	~	~	~			
Forest Ave Westbound	**	**	**	~	~	~	~	~	~	~	~	~			
Forest Ave Eastbound	**	**	**	~	~	~	~	~	~	~	~	~			
*Nason Road	~	~	~	~	~	~	~	~	~	~	~	~			

1 - Volume to Capacity Ratio

2 - Average Delay expressed in seconds per vehicle

3 - Level of Service

~ - Indicates that the intersection is operating as free flowing.

\* - In the AM and PM peak hrs 4 and 25 cars, respectively dropped/picked up students. This is an enforcement issue.

\*\* - 4/5 cars were observed queuing w/ 20 second delays, however the analysis showed no delay or queuing should occur if students were not being dropped off on the road.

The western driveway operates in a satisfactory condition in the existing and proposed conditions. The middle driveway analysis shows the existing Forest Ave movements and the northbound driveway (entrance into the MS) with LOS E and D. The proposed condition with the Elementary School shows Forest Ave. east and westbound free flowing with no delays, however the northbound movement operates at a LOS F. This is because the delayed vehicles on Forest Ave. are now on site in the expanded MS drop off/pick up location.

The same condition exists for Sargent Road. Opening up the eastern driveway and prohibiting drop offs on Sargent St will move the vehicles from Sargent Rd onto the school site, via the eastern driveway.

### **EXECUTIVE SUMMARY**

The study area roadways, school driveways and Forest Ave / Sargent Road intersection operate at a LOS A, except during the 15 - 20 minute period before school begins and ends. During this peak 15 – 20 minutes approximately 300 vehicles and 175 pedestrians operate in the study area to populate the MS. Additionally, another approximately 300 vehicles, having no business with the MS pass through the study area. We believe the additional traffic is predominately using the study area to by pass the congestion on Humphrey Street (Rte 129). The combination of this traffic, school and non school related, is causing the delays and queueing on Forest Ave. and Sargent Road.

The ES will attract approximately 220 and 133 vehicles in the AM and PM peak periods, respectively. However an additional access/egress point will be added by connecting Nason Rd to the ES, and the eastern driveway will be opened to provide greater access and egress. Additionally the main access driveway to the MS will be expanded providing greater drop off and queuing availability.

The ES will start classes a minimum of 20 minutes after the MS. Based on our numerous observations the MS traffic will be dispersed at or close to 8am. The ES traffic will then begin to arrive and the drop off process will again run it's course from 8:00am – 8:20am. The analysis prepared for this TIS assumes a worse case scenario, that being both schools start at the same time. Our approach in this report is that if we can manage all of the drop offs, happening at the same time, on site we will have alleviated the congestion on the local street network. We have done this by designing three entrance/exit driveways with extensive drop off/pick up locations.

The following is a discussion regarding the analytical data, and considering observed operating conditions in the study area.

#### Forest Ave. – Western (Exit) Driveway

The western driveway experiences queuing in the AM primarily because exiting vehicles are attempting to head eastbound on Forest Ave. to bring students to the Stanley ES. The queuing on Forest Ave eastbound resulting from the Middle driveway inhibits the left turn out of the western driveway. Adding the ES to the site should significantly reduce the need for this movement, and requiring parents to drop off on site should reduce the queue on Forest Ave. eastbound.

### Forest Ave. – Middle (Entrance) Driveway

The middle driveway provides access to the MS and curbside drop off on site. The existing combination of limited curbside length for drop off, drop offs occurring on both sides of Forest Ave from both eastbound and westbound traffic, and the length of time the crossing guard is taking to process students, all of this is contributing to the queues on Forest Ave from both directions. The proposed design significantly increases the curb side drop off, the enforcement of prohibiting drop offs on Forest Ave, and the re-training of the crossing guard to consider processing vehicles as well as pedestrians should significantly reduce the congestion and queues on Forest Ave.

### Forest Ave. – Eastern (Exit / Entrance) Driveway

The eastern driveway is presently underutilized and experiences no delays because it does not provide a drop off condition that the parents find useful. The proposed condition expands the eastern driveway to provide access to the ES, the MS and egress to Nason Rd and Forest Ave. at the western driveway. We expect expansion of the eastern driveway's capabilities should significantly mitigate additional traffic generated by the ES.

### Forest Ave. / Sargent Road

Forest Ave, eastbound and westbound queue to approximately 4 - 5 vehicles and Sargent Road queues to approximately 5 - 6 vehicles based on field observations. The delay, or time it takes for a vehicle to get through the intersection, is approximately 30 seconds. The queuing and delay are largely a result of student drop offs on Sargent Rd and the crossing guard processing the students across both Sargent Rd and Forest Ave. Due to the expanded use of the eastern driveway parents will be able to drop off their students more conveniently and safely on site, resulting in less congestion at the intersection. Enforcement and signing will be helpful to encourage the use of the eastern driveway.

### Nason Road

Nason Road presently processes approximately 4 drop offs in the AM peak hour and 25 pick ups in the PM peak hour. The parents pull of the road near the fenced area at the end of Nason Road which abuts the school site. In the future Nason Road will be extended into a new school driveway leading to curb side drop off/pick up in front of the new ES. This new point of access/egress to the site will provide extensive curbside drop-offs and queuing length on school property. Additionally the parents will be able to exit via Nason Rd and Forest Ave on the other side of the school property at the western, exit driveway. By creating the egress movement to the western driveway vehicles have been taken off the roadway from Nason Rd all the way to Forest Ave, beyond the school. This means the vehicle will not process through the Sargent Rd/Forest Ave intersection, or the eastern and western driveways.

Several issues contribute to the existing congestion. Those issues, and the issues which arise from the additional ES trips can be mitigated through design, enforcement and education. The following is an itemization of remedies which contribute to alleviating the congestion and queueing on the study area roadways:

1. The design of the new ES includes providing a drop off location with significant queue length on school property from Nason Rd.

- 2. The front entrance to the MS will be widened and lengthened to provide a longer drop off and queue length on school property.
- 3. The eastern driveway will be widened and extended to provide drop off locations for both the ES and MS. After the drop offs/pick ups occur the driver will be given the option to exit the school site via Nason Rd to the east, back out to Forest Ave in the same location they came in, or circle around the schools and exit out to Forest Ave from the western driveway.
- 4. The analysis assumed the MS and ES would be operating in the same peak period, however the schools presently open twenty minutes apart, the MS starts at 8:00am and the ES at 8:20am. The congestion and queues generally take place 15 20 minutes before school starts. If the schools start/end 20 30 minutes apart from each other the congestion and queues will not overlap, and consequently be more manageable.
- 5. Presently the crossing guards are providing a valuable service by shepherding students across the cross walks at the main access driveway and across Sargent Road/Forest Ave. The guards could be trained to facilitate the traffic in combination with safeguarding the student movements.
- 6. Signs and enforcement should be provided on Forest Ave, Laurel Rd, Nason Rd and Sargent Rd which restricts the parents from dropping off the students on the study area roadways. Students must be brought into the school site, and then dropped off / picked up. This will take the drop off movement from the study area roadways and allow traffic to move more freely.
- 7. As described above Forest Ave is used as a cut through to bypass the heavily congested Humphrey St. This situation accounts for approximately 50% of the traffic on Forest Ave in both peak hours, and more significantly in the AM. The cut through could be redirected, and this would significantly reduce the number of vehicles in the study area during the peak periods.
- 8. The school district is contemplating an increase use of buses. A single bus will take approximately 65 students, which will result in approximately 30 less vehicles coming to the school. Industry traffic analysis software cannot accurately measure the reduction in delay and queuing which results from 30 less vehicles, however we expect approximately 520 vehicles to come to the site (albeit over two peak periods, 7:40 8am and 8 8:20am) so we can assume four buses will result in a little less than 25% reduction in delay and queue.

It is GEOD's professional opinion that the study area roadway network should be able to process the existing Middle School and proposed Elementary School due to the design features, enforcement, education and staggered start times of the two schools. Redistributing the student drop-off location from Forest Ave. to on-site locations has the potential of reducing congestion within the study area network The list of remedies above should provide adequate mitigation to process the existing and proposed traffic.

# **APPENDIX A**

TRAFFIC COUNT DATA

N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

	SMS	Westerly Exit	Only Drivewa	у		Forest Av	venue Fast			Forest Av	venue Vest		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	n 07:00 AM to 0	8:45 AM - Pea	k 1 of 1										
Peak Hour for Entire Inte	rsection Begins	at 07:30 AM											
07:30 AM	31	12	22	65	0	47	0	47	81	0	1	82	194
07.43 AM 08:00 AM	19	21	1	28	0	20	0	20	49	0	0	49	299 97
08:15 AM	9	6	5	20	0	29	ő	29	47	0	0	47	96
Total Volume	147	47	58	252	0	134	0	134	299	0	1	300	686
% App. Total	58.3	18.7	23		0	100	0		99.7	0	0.3		
PHF	.418	.560	.483	.453	.000	.713	.000	.713	.613	.000	.250	.615	.574
% Cars & Peds	147	100	983	99.6	0	96.3	0	96.3	993	0	0	99.0	98.7
Trucks & Bikes	0	0	1	1	0	1	Ő	1	1	0	1	2	4
% Trucks & Bikes	0	0	1.7	0.4	0	0.7	0	0.7	0.3	0	100	0.7	0.6
School Buses	0	0	0	0	0	4	0	4	1	0	0	1	5
% School Buses	0	0	0	0	0	3.0	0	3.0	0.3	0	0	0.3	0.7
	Forest Avenue	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			SMS Wes Out 0 0 0 1 1 Rig • Peak Hour Cars & Pec Trucks & B School Bus	terly Exit Or 251 1 0 252 47 47 0 0 0 0 47 47 47 0 0 0 0 47 47 47 0 0 0 0 0 47 47 47 North Begins at 0 <sup>o</sup> ds ikes ises	Total 251 0 252 57 1 0 58 Peds 7 Data		Right Thru Peds	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Out In Total		

N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name	: 04404A
Site Code	: 04404
Start Date	: 4/1/2014
Page No	: 1

			Groups Printe	d- Cars & Peds -	Trucks & Bikes -	School Buses				
	SMS West	terly Exit Only D	riveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	4	0	0	0	14	0	12	0	0	30
07:15 AM	4	2	5	0	29	0	39	0	0	79
07:30 AM	31	12	22	0	47	0	81	0	1	194
07:45 AM	88	21	30	0	38	0	122	0	0	299
Total	127	35	57	0	128	0	254	0	1	602
08:00 AM	19	8	1	0	20	0	49	0	0	97
08:15 AM	9	6	5	0	29	0	47	0	0	96
08:30 AM	20	7	0	0	24	0	24	0	0	75
08:45 AM	7	2	0	0	12	0	11	0	0	32
Total	55	23	6	0	85	0	131	0	0	300
Grand Total	182	58	63	0	213	0	385	0	1	902
Apprch %	60.1	19.1	20.8	0	100	0	99.7	0	0.3	
Total %	20.2	6.4	7	0	23.6	0	42.7	0	0.1	
Cars & Peds	181	58	59	0	205	0	382	0	0	885
% Cars & Peds	99.5	100	93.7	0	96.2	0	99.2	0	0	98.1
Trucks & Bikes	0	0	4	0	2	0	2	0	1	9
% Trucks & Bikes	0	0	6.3	0	0.9	0	0.5	0	100	1
School Buses	1	0	0	0	6	0	1	0	0	8
% School Buses	0.5	0	0	0	2.8	0	0.3	0	0	0.9

	SMS	Westerly Exit	Only Drive	eway	Forest Avenue				Forest Avenue				
		From N	Jorth			From	East			From '	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 0	8:45 AM - Pea	ak 1 of 1										
Peak Hour for Entire Inter-	section Begins	at 07:30 AM											
07:30 AM	31	12	22	65	0	47	0	47	81	0	1	82	194
07:45 AM	88	21	30	139	0	38	0	38	122	0	0	122	299
08:00 AM	19	8	1	28	0	20	0	20	49	0	0	49	97
08:15 AM	9	6	5	20	0	29	0	29	47	0	0	47	96
Total Volume	147	47	58	252	0	134	0	134	299	0	1	300	686
% App. Total	58.3	18.7	23		0	100	0		99.7	0	0.3		
PHF	.418	.560	.483	.453	.000	.713	.000	.713	.613	.000	.250	.615	.574
Cars & Peds	147	47	57	251	0	129	0	129	297	0	0	297	677
% Cars & Peds	100	100	98.3	99.6	0	96.3	0	96.3	99.3	0	0	99.0	98.7
Trucks & Bikes	0	0	1	1	0	1	0	1	1	0	1	2	4
% Trucks & Bikes	0	0	1.7	0.4	0	0.7	0	0.7	0.3	0	100	0.7	0.6
School Buses	0	0	0	0	0	4	0	4	1	0	0	1	5
% School Buses	0	0	0	0	0	3.0	0	3.0	0.3	0	0	0.3	0.7

N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

: 04404A
: 04404
: 4/1/2014
:1

Groups Printed- Cars & Peds												
	SMS West	erly Exit Only Dr	iveway		Forest Avenue							
		From North			From East			From West				
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total		
07:00 AM	4	0	0	0	13	0	12	0	0	29		
07:15 AM	4	2	2	0	27	0	39	0	0	74		
07:30 AM	31	12	22	0	45	0	80	0	0	190		
07:45 AM	88	21	30	0	36	0	122	0	0	297		
Total	127	35	54	0	121	0	253	0	0	590		
08:00 AM	19	8	1	0	20	0	49	0	0	97		
08:15 AM	9	6	4	0	28	0	46	0	0	93		
08:30 AM	19	7	0	0	24	0	24	0	0	74		
08:45 AM	7	2	0	0	12	0	10	0	0	31		
Total	54	23	5	0	84	0	129	0	0	295		
Grand Total	181	58	59	0	205	0	382	0	0	885		
Apprch %	60.7	19.5	19.8	0	100	0	100	0	0			
Total %	20.5	6.6	6.7	0	23.2	0	43.2	0	0			

	SMS	Westerly Exit	Only Drive	eway	Forest Avenue				Forest Avenue				
		From N	North			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 08	3:45 AM - Pea	ak 1 of 1										
Peak Hour for Entire Inter-	section Begins a	at 07:30 AM											
07:30 AM	31	12	22	65	0	45	0	45	80	0	0	80	190
07:45 AM	88	21	30	139	0	36	0	36	122	0	0	122	297
08:00 AM	19	8	1	28	0	20	0	20	49	0	0	49	97
08:15 AM	9	6	4	19	0	28	0	28	46	0	0	46	93
Total Volume	147	47	57	251	0	129	0	129	297	0	0	297	677
% App. Total	58.6	18.7	22.7		0	100	0		100	0	0		
PHF	.418	.560	.475	.451	.000	.717	.000	.717	.609	.000	.000	.609	.570

N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

				Groups Printed	d- Trucks & Bikes	8				
	SMS West	terly Exit Only D	riveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	1	0	0	0	0	1
07:15 AM	0	0	3	0	0	0	0	0	0	3
07:30 AM	0	0	0	0	0	0	1	0	1	2
07:45 AM	0	0	0	0	0	0	0	0	0	0
Total	0	0	3	0	1	0	1	0	1	6
08:00 AM	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	1	0	1	0	0	0	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	1	0	0	1
Total	0	0	1	0	1	0	1	0	0	3
Grand Total	0	0	4	0	2	0	2	0	1	9
Apprch %	0	0	100	0	100	0	66.7	0	33.3	
Total %	0	0	44.4	0	22.2	0	22.2	0	11.1	

	SMS	Westerly Exit	t Only Drive	eway		Forest	Avenue			Forest	Avenue		
		From 1	North			From	n East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 08	8:45 AM - Pe	ak 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 07:00 AM											
07:00 AM	0	0	0	0	0	1	0	1	0	0	0	0	1
07:15 AM	0	0	3	3	0	0	0	0	0	0	0	0	3
07:30 AM	0	0	0	0	0	0	0	0	1	0	1	2	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	3	3	0	1	0	1	1	0	1	2	6
% App. Total	0	0	100		0	100	0		50	0	50		
PHF	.000	.000	.250	.250	.000	.250	.000	.250	.250	.000	.250	.250	.500

N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

				Groups Printe	d- School Buses					
	SMS West	erly Exit Only Di	riveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	2	0	0	0	0	2
07:30 AM	0	0	0	0	2	0	0	0	0	2
07:45 AM	0	0	0	0	2	0	0	0	0	2
Total	0	0	0	0	6	0	0	0	0	6
08:00 AM	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	1	0	0	1
08:30 AM	1	0	0	0	0	0	0	0	0	1
08:45 AM	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	0	0	0	1	0	0	2
Grand Total	1	0	0	0	6	0	1	0	0	8
Apprch %	100	0	0	0	100	0	100	0	0	
Total %	12.5	0	0	0	75	0	12.5	0	0	

	SMS	Westerly Exit	Only Drive	eway		Forest	Avenue			Forest .	Avenue		
		From N	North			From	n East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 0	8:45 AM - Pe	ak 1 of 1										
Peak Hour for Entire Inter-	section Begins	at 07:00 AM											
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
07:30 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
07:45 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
Total Volume	0	0	0	0	0	6	0	6	0	0	0	0	6
% App. Total	0	0	0		0	100	0		0	0	0		
PHF	.000	.000	.000	.000	.000	.750	.000	.750	.000	.000	.000	.000	.750

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

	SMS	Middle Enter	Only Drivewa	vay Forest Avenue Forest Avenue									
Start Time	Right	From N	Orth Peds	App. Total	Right	From E	East	App. Total	Thru	From W	Peds	App. Total	Int Total
Peak Hour Analysis From	07:00 AM to 0	8:45 AM - Pea	k 1 of 1	App. Total	Right	Tinu	Teus	App. Total	Intu	Luit	Teus	App. Total	Int. Total
Peak Hour for Entire Inters	section Begins	at 07:30 AM											
07:30 AM	0	0	30	30	30	48	0	78	52	40	9	101	209
07:45 AM 08:00 AM	0	0	56	56	62 10	40	0	102	59 40	83	29	1/1	329
08:15 AM	0	0	8	8	11	29	0	40	33	19	3	55	103
Total Volume	0	0	95	95	122	136	0	258	193	149	41	383	736
% App. Total	0	0	100	424	47.3	52.7	0	(22)	50.4	38.9	10.7	5(0)	550
Cars & Peds	.000	000	94	.424 94	.492	131	000.	252	<u>.818</u> 191	149	40	.560	.339
% Cars & Peds	0	0	98.9	98.9	99.2	96.3	0	97.7	99.0	100	97.6	99.2	98.6
Trucks & Bikes	0	0	1	1	0	1	0	1	1	0	1	2	4
% Irucks & Bikes School Buses	0	0	1.1	1.1	0	0.7	0	0.4	0.5	0	2.4	0.5	0.5
% School Buses	0	0	0	0	0.8	2.9	0	1.9	0.5	0	0	0.3	0.8
					SMS Mid	dle Enter Or	nly Driveway						
					Out		Total						
					2/0	94	304						
					1	0	1						
					271	95	366						
					l r								
						0 0	94						
							1						
						0 0	95						
					Rig	ht Left	Peds						
						$\rightarrow$							
					Peak	k Hour	Data						
	-	<u>1003</u>									- I		
		2	0 0 0 0	. <b>.</b>		T		<b>A</b>	7		P		
	<u>e</u>		4 4			North		L	in 12 12		, <del>,</del> ,		
	enu	33-1-2 30			Peak Hour	Begins at 0	7:30 AM		$+ \underline{N} \rightarrow 0 \rightarrow$		ore		
	¥-	= <u>₩</u> ₩_	10, 10					←			In St A		
	est			i -	Cars & Peo Trucks & B	js likes		2	<sup>2</sup> 36 4 1 31	<u>68</u> 5 → N	ver		
	For	<u> </u>	6-04	- S	School Bus	Ses		-		]	ue l		
	Ċ	3~ ~		Pe				ġ	0000	4 4	Tot		
				-						51 6 2 4	<u>a</u>		

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

		G	roups Printed- (	Cars & Peds - Truc	ks & Bikes - Sch	ool Buses				
	SMS Middle E	Enter Only Drivev	vay	For	est Avenue		Fore	st Avenue		
	Fr	om North		F	rom East		Fr	om West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	2	5	14	0	5	7	2	35
07:15 AM	0	0	7	13	29	0	25	16	2	92
07:30 AM	0	0	30	30	48	0	52	40	9	209
07:45 AM	0	0	56	62	40	0	59	83	29	329
Total	0	0	95	110	131	0	141	146	42	665
08:00 AM	0	0	1	19	19	0	49	7	0	95
08:15 AM	0	0	8	11	29	0	33	19	3	103
08:30 AM	0	0	0	7	25	0	23	8	0	63
08:45 AM	0	0	0	2	10	0	12	1	0	25
Total	0	0	9	39	83	0	117	35	3	286
Grand Total	0	0	104	149	214	0	258	181	45	951
Apprch %	0	0	100	41	59	0	53.3	37.4	9.3	
Total %	0	0	10.9	15.7	22.5	0	27.1	19	4.7	
Cars & Peds	0	0	100	148	206	0	255	181	44	934
% Cars & Peds	0	0	96.2	99.3	96.3	0	98.8	100	97.8	98.2
Trucks & Bikes	0	0	4	0	2	0	2	0	1	9
% Trucks & Bikes	0	0	3.8	0	0.9	0	0.8	0	2.2	0.9
School Buses	0	0	0	1	6	0	1	0	0	8
% School Buses	0	0	0	0.7	2.8	0	0.4	0	0	0.8

	SMS	Middle Enter	Only Drive	way		Forest A	Avenue			Forest A	venue		
		From N	North			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 0	8:45 AM - Pea	ak 1 of 1										
Peak Hour for Entire Inters	section Begins	at 07:30 AM											
07:30 AM	0	0	30	30	30	48	0	78	52	40	9	101	209
07:45 AM	0	0	56	56	62	40	0	102	59	83	29	171	329
08:00 AM	0	0	1	1	19	19	0	38	49	7	0	56	95
08:15 AM	0	0	8	8	11	29	0	40	33	19	3	55	103
Total Volume	0	0	95	95	122	136	0	258	193	149	41	383	736
% App. Total	0	0	100		47.3	52.7	0		50.4	38.9	10.7		
PHF	.000	.000	.424	.424	.492	.708	.000	.632	.818	.449	.353	.560	.559
Cars & Peds	0	0	94	94	121	131	0	252	191	149	40	380	726
% Cars & Peds	0	0	98.9	98.9	99.2	96.3	0	97.7	99.0	100	97.6	99.2	98.6
Trucks & Bikes	0	0	1	1	0	1	0	1	1	0	1	2	4
% Trucks & Bikes	0	0	1.1	1.1	0	0.7	0	0.4	0.5	0	2.4	0.5	0.5
School Buses	0	0	0	0	1	4	0	5	1	0	0	1	6
% School Buses	0	0	0	0	0.8	2.9	0	1.9	0.5	0	0	0.3	0.8

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

				Groups Print	ed- Cars & Peds					
	SMS Mide	dle Enter Only Dr	iveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	2	5	13	0	5	7	2	34
07:15 AM	0	0	4	13	27	0	25	16	2	87
07:30 AM	0	0	30	30	46	0	51	40	8	205
07:45 AM	0	0	56	62	38	0	59	83	29	327
Total	0	0	92	110	124	0	140	146	41	653
08:00 AM	0	0	1	19	19	0	49	7	0	95
08:15 AM	0	0	7	10	28	0	32	19	3	99
08:30 AM	0	0	0	7	25	0	23	8	0	63
08:45 AM	0	0	0	2	10	0	11	1	0	24
Total	0	0	8	38	82	0	115	35	3	281
Grand Total	0	0	100	148	206	0	255	181	44	934
Apprch %	0	0	100	41.8	58.2	0	53.1	37.7	9.2	
Total %	0	0	10.7	15.8	22.1	0	27.3	19.4	4.7	

	SMS	Middle Enter	r Only Drive	way		Forest A	Avenue			Forest A	Avenue		
		From	North			From	n East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 08	8:45 AM - Pe	ak 1 of 1										
Peak Hour for Entire Inter-	section Begins a	at 07:30 AM											
07:30 AM	0	0	30	30	30	46	0	76	51	40	8	99	205
07:45 AM	0	0	56	56	62	38	0	100	59	83	29	171	327
08:00 AM	0	0	1	1	19	19	0	38	49	7	0	56	95
08:15 AM	0	0	7	7	10	28	0	38	32	19	3	54	99
Total Volume	0	0	94	94	121	131	0	252	191	149	40	380	726
% App. Total	0	0	100		48	52	0		50.3	39.2	10.5		
PHF	.000	.000	.420	.420	.488	.712	.000	.630	.809	.449	.345	.556	.555

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

				Groups Printee	d- Trucks & Bike	s				
	SMS Mide	dle Enter Only Dr	iveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	1	0	0	0	0	1
07:15 AM	0	0	3	0	0	0	0	0	0	3
07:30 AM	0	0	0	0	0	0	1	0	1	2
07:45 AM	0	0	0	0	0	0	0	0	0	0
Total	0	0	3	0	1	0	1	0	1	6
08:00 AM	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	1	0	1	0	0	0	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	1	0	0	1
Total	0	0	1	0	1	0	1	0	0	3
Grand Total	0	0	4	0	2	0	2	0	1	9
Apprch %	0	0	100	0	100	0	66.7	0	33.3	
Total %	0	0	44.4	0	22.2	0	22.2	0	11.1	

	SMS	Middle Enter	Only Drive	eway		Forest	Avenue			Forest	Avenue		
		From 1	North			Fron	n East			From	n West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 08	8:45 AM - Pe	ak 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 07:00 AM											
07:00 AM	0	0	0	0	0	1	0	1	0	0	0	0	1
07:15 AM	0	0	3	3	0	0	0	0	0	0	0	0	3
07:30 AM	0	0	0	0	0	0	0	0	1	0	1	2	2
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	3	3	0	1	0	1	1	0	1	2	6
% App. Total	0	0	100		0	100	0		50	0	50		
PHF	.000	.000	.250	.250	.000	.250	.000	.250	.250	.000	.250	.250	.500

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

				Groups Printe	ed- School Buses					
	SMS Mide	dle Enter Only Dr	iveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	2	0	0	0	0	2
07:30 AM	0	0	0	0	2	0	0	0	0	2
07:45 AM	0	0	0	0	2	0	0	0	0	2
Total	0	0	0	0	6	0	0	0	0	6
08:00 AM	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	1	0	0	1	0	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	1	0	0	2
Grand Total	0	0	0	1	6	0	1	0	0	8
Apprch %	0	0	0	14.3	85.7	0	100	0	0	
Total %	0	0	0	12.5	75	0	12.5	0	0	

	SMS	Middle Enter	Only Drive	way		Forest A	Avenue			Forest A	Avenue		
		From N	lorth			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	07:00 AM to 08	:45 AM - Pea	ak 1 of 1										
Peak Hour for Entire Inters	section Begins a	t 07:00 AM											
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
07:30 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
07:45 AM	0	0	0	0	0	2	0	2	0	0	0	0	2
Total Volume	0	0	0	0	0	6	0	6	0	0	0	0	6
% App. Total	0	0	0		0	100	0		0	0	0		
PHF	.000	.000	.000	.000	.000	.750	.000	.750	.000	.000	.000	.000	.750

## **Transportation Data Corporation**

Mario Perone, mperone1@verizon.net tel (781) 587-0086 cell (781) 439-4999

N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

		SMS Ea	sterly D	riveway			For	rest Aver	nue			Sa	rgent Ro	ad			For	est Aver	nue		
		F	rom Nor	th			I	From East	st			F	rom Sou	th			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From (	07:00 AM	l to 08:45	AM - P	eak 1 of 1																
Peak Hour for Ent	ire Inters	ection Be	gins at 0	7:30 AM																	
07:30 AM	2	1	0	15	18	1	38	8	0	47	4	9	34	9	56	15	29	5	39	88	209
07:45 AM	0	0	1	31	32	2	56	7	0	65	9	4	41	13	67	27	29	0	38	94	258
08:00 AM	1	0	0	0	1	2	23	2	2	29	12	1	12	3	28	3	47	2	0	52	110
08:15 AM	1	1	0	5	7	0	34	2	0	36	6	0	8	3	17	10	22	2	3	37	97
Total Volume	4	2	1	51	58	5	151	19	2	177	31	14	95	28	168	55	127	9	80	271	674
% App. Total	6.9	3.4	1.7	87.9		2.8	85.3	10.7	1.1		18.5	8.3	56.5	16.7		20.3	46.9	3.3	29.5		
PHF	.500	.500	.250	.411	.453	.625	.674	.594	.250	.681	.646	.389	.579	.538	.627	.509	.676	.450	.513	.721	.653
Cars & Peds	4	2	1	51	58	5	149	19	2	175	31	14	91	28	164	55	125	9	80	269	666
% Cars & Peds	100	100	100	100	100	100	98.7	100	100	98.9	100	100	95.8	100	97.6	100	98.4	100	100	99.3	98.8
Trucks & Bikes	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
% Trucks & Bikes	0	0	0	0	0	0	0.7	0	0	0.6	0	0	0	0	0	0	0.8	0	0	0.4	0.3
School Buses	0	0	0	0	0	0	1	0	0	1	0	0	4	0	4	0	1	0	0	1	6
% School Buses	0	0	0	0	0	0	0.7	0	0	0.6	0	0	4.2	0	2.4	0	0.8	0	0	0.4	0.9



N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

					Grou	ps Printed-	Cars & Peo	is - Trucks	& Bikes - S	School Bus	es						
	SM	S Easterly	Driveway			Forest Av	/enue			Sargent F	Road			Forest Av	enue		
		From No	orth			From E	East			From Se	outh			From W	Vest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	1	0	0	0	3	8	1	0	2	0	9	0	0	3	1	0	28
07:15 AM	2	0	2	3	1	26	2	0	1	1	15	4	3	16	6	3	85
07:30 AM	2	1	0	15	1	38	8	0	4	9	34	9	15	29	5	39	209
07:45 AM	0	0	1	31	2	56	7	0	9	4	41	13	27	29	0	38	258
Total	5	1	3	49	7	128	18	0	16	14	99	26	45	77	12	80	580
08:00 AM	1	0	0	0	2	23	2	2	12	1	12	3	3	47	2	0	110
08:15 AM	1	1	0	5	0	34	2	0	6	0	8	3	10	22	2	3	97
08:30 AM	1	0	0	0	2	17	1	0	4	1	12	2	6	16	0	0	62
08:45 AM	1	0	0	0	0	5	3	0	5	0	6	1	6	5	2	0	34
Total	4	1	0	5	4	79	8	2	27	2	38	9	25	90	6	3	303
Grand Total	9	2	3	54	11	207	26	2	43	16	137	35	70	167	18	83	883
Apprch %	13.2	2.9	4.4	79.4	4.5	84.1	10.6	0.8	18.6	6.9	59.3	15.2	20.7	49.4	5.3	24.6	
Total %	1	0.2	0.3	6.1	1.2	23.4	2.9	0.2	4.9	1.8	15.5	4	7.9	18.9	2	9.4	
Cars & Peds	8	2	3	53	11	205	25	2	43	16	131	35	69	165	18	83	869
% Cars & Peds	88.9	100	100	98.1	100	99	96.2	100	100	100	95.6	100	98.6	98.8	100	100	98.4
Trucks & Bikes	1	0	0	1	0	1	1	0	0	0	0	0	1	1	0	0	6
% Trucks & Bikes	11.1	0	0	1.9	0	0.5	3.8	0	0	0	0	0	1.4	0.6	0	0	0.7
School Buses	0	0	0	0	0	1	0	0	0	0	6	0	0	1	0	0	8
% School Buses	0	0	0	0	0	0.5	0	0	0	0	4.4	0	0	0.6	0	0	0.9

		SMS Ea	sterly Dr	iveway			For	rest Aver	nue			Sa	rgent Ro	ad			Fo	rest Avei	nue		
		F	rom Nor	th			I	From Eas	t			F	From Sou	th			F	From We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From 0	7:00 AM	to 08:45	AM - P	eak 1 of 1																
Peak Hour for Ent	ire Interse	ection Beg	gins at 07	7:30 AM																	
07:30 AM	2	1	0	15	18	1	38	8	0	47	4	9	34	9	56	15	29	5	39	88	209
07:45 AM	0	0	1	31	32	2	56	7	0	65	9	4	41	13	67	27	29	0	38	94	258
08:00 AM	1	0	0	0	1	2	23	2	2	29	12	1	12	3	28	3	47	2	0	52	110
08:15 AM	1	1	0	5	7	0	34	2	0	36	6	0	8	3	17	10	22	2	3	37	97
Total Volume	4	2	1	51	58	5	151	19	2	177	31	14	95	28	168	55	127	9	80	271	674
% App. Total	6.9	3.4	1.7	87.9		2.8	85.3	10.7	1.1		18.5	8.3	56.5	16.7		20.3	46.9	3.3	29.5		
PHF	.500	.500	.250	.411	.453	.625	.674	.594	.250	.681	.646	.389	.579	.538	.627	.509	.676	.450	.513	.721	.653
Cars & Peds	4	2	1	51	58	5	149	19	2	175	31	14	91	28	164	55	125	9	80	269	666
% Cars & Peds	100	100	100	100	100	100	98.7	100	100	98.9	100	100	95.8	100	97.6	100	98.4	100	100	99.3	98.8
Trucks & Bikes	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1	2
% Trucks & Bikes	0	0	0	0	0	0	0.7	0	0	0.6	0	0	0	0	0	0	0.8	0	0	0.4	0.3
School Buses	0	0	0	0	0	0	1	0	0	1	0	0	4	0	4	0	1	0	0	1	6
% School Buses	0	0	0	0	0	0	0.7	0	0	0.6	0	0	4.2	0	2.4	0	0.8	0	0	0.4	0.9

N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name	: 04404C
Site Code	: 04404
Start Date	: 4/1/2014
Page No	: 1

							Groups P	rinted- Ca	rs & Peds								
	SM	S Easterly	Driveway			Forest A	venue			Sargent F	Road			Forest A	venue		
		From N	orth			From I	East			From Se	outh			From V	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	3	8	1	0	2	0	9	0	0	3	1	0	27
07:15 AM	2	0	2	2	1	26	1	0	1	1	13	4	3	16	6	3	81
07:30 AM	2	1	0	15	1	38	8	0	4	9	31	9	15	28	5	39	205
07:45 AM	0	0	1	31	2	56	7	0	9	4	40	13	27	29	0	38	257
Total	4	1	3	48	7	128	17	0	16	14	93	26	45	76	12	80	570
08:00 AM	1	0	0	0	2	23	2	2	12	1	12	3	3	47	2	0	110
08:15 AM	1	1	0	5	0	32	2	0	6	0	8	3	10	21	2	3	94
08:30 AM	1	0	0	0	2	17	1	0	4	1	12	2	6	16	0	0	62
08:45 AM	1	0	0	0	0	5	3	0	5	0	6	1	5	5	2	0	33
Total	4	1	0	5	4	77	8	2	27	2	38	9	24	89	6	3	299
Grand Total	8	2	3	53	11	205	25	2	43	16	131	35	69	165	18	83	869
Apprch %	12.1	3	4.5	80.3	4.5	84.4	10.3	0.8	19.1	7.1	58.2	15.6	20.6	49.3	5.4	24.8	
Total %	0.9	0.2	0.3	6.1	1.3	23.6	2.9	0.2	4.9	1.8	15.1	4	7.9	19	2.1	9.6	

		SMS Ea	sterly D	riveway			For	est Aver	nue			Sa	rgent Ro	ad			For	est Aver	nue		]
		F	rom Nor	th			F	rom Eas	st			F	rom Sou	th			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	sis From (	07:00 AM	to 08:45	5 AM - P	eak 1 of 1																
Peak Hour for Ent	tire Inters	ection Be	gins at 0	7:30 AM																	
07:30 AM	2	1	0	15	18	1	38	8	0	47	4	9	31	9	53	15	28	5	39	87	205
07:45 AM	0	0	1	31	32	2	56	7	0	65	9	4	40	13	66	27	29	0	38	94	257
08:00 AM	1	0	0	0	1	2	23	2	2	29	12	1	12	3	28	3	47	2	0	52	110
08:15 AM	1	1	0	5	7	0	32	2	0	34	6	0	8	3	17	10	21	2	3	36	94
Total Volume	4	2	1	51	58	5	149	19	2	175	31	14	91	28	164	55	125	9	80	269	666
% App. Total	6.9	3.4	1.7	87.9		2.9	85.1	10.9	1.1		18.9	8.5	55.5	17.1		20.4	46.5	3.3	29.7		
PHF	.500	.500	.250	.411	.453	.625	.665	.594	.250	.673	.646	.389	.569	.538	.621	.509	.665	.450	.513	.715	.648

N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name	: 04404C
Site Code	: 04404
Start Date	: 4/1/2014
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							Groups Pri	inted- Truc	ks & Bikes								
	SM	S Easterly	Driveway			Forest A	venue			Sargent F	Road			Forest Av	enue		
		From N	lorth			From I	East			From So	outh			From W	/est		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 AM	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	2
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	1	0	0	1	0	0	1	0	0	0	0	0	0	1	0	0	4
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1
Total	0	0	0	0	0	1	0	0	0	0	0	0	1	0	0	0	2
Grand Total	1	0	0	1	0	1	1	0	0	0	0	0	1	1	0	0	6
Apprch %	50	0	0	50	0	50	50	0	0	0	0	0	50	50	0	0	
Total %	16.7	0	0	16.7	0	16.7	16.7	0	0	0	0	0	16.7	16.7	0	0	

		SMS Ea	sterly D	riveway			For	est Aver	nue			Sa	rgent Ro	ad			For	est Aver	ue		
		F	rom Nor	th			F	rom Eas	st			F	rom Sou	th			F	rom Wes	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	sis From (	07:00 AM	to 08:45	AM - P	eak 1 of 1																
Peak Hour for En	tire Inters	ection Be	gins at 0'	7:00 AM																	
07:00 AM	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07:15 AM	0	0	0	1	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	2
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	1	0	0	1	2	0	0	1	0	1	0	0	0	0	0	0	1	0	0	1	4
% App. Total	50	0	0	50		0	0	100	0		0	0	0	0		0	100	0	0		
PHF	.250	.000	.000	.250	.500	.000	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250	.500

N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name	: 04404C
Site Code	: 04404
Start Date	: 4/1/2014
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							Groups Pr	rinted- Sch	ool Buses								
	SM	S Easterly	Driveway			Forest Av	venue			Sargent F	Road			Forest Av	enue		
		From N	orth			From H	East			From Se	outh			From V	/est		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	2
07:30 AM	0	0	0	0	0	0	0	0	0	0	3	0	0	0	0	0	3
07:45 AM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
Total	0	0	0	0	0	0	0	0	0	0	6	0	0	0	0	0	6
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	2
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	2
Grand Total	0	0	0	0	0	1	0	0	0	0	6	0	0	1	0	0	8
Apprch %	0	0	0	0	0	100	0	0	0	0	100	0	0	100	0	0	
Total %	0	0	0	0	0	12.5	0	0	0	0	75	0	0	12.5	0	0	

	SMS Easterly Driveway					Forest Avenue				Sargent Road				Forest Avenue							
	From North					From East				From South				From West							
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	sis From (	07:00 AM	to 08:45	5 AM - P	eak 1 of 1																
Peak Hour for En	tire Inters	ection Be	gins at 0	7:00 AM																	
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	0	0	2
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	0	0	0	0	0	3
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1
Total Volume	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	0	0	0	0	0	6
% App. Total	0	0	0	0		0	0	0	0		0	0	100	0		0	0	0	0		
PHF	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.500	.000	.500	.000	.000	.000	.000	.000	.500

# Transportation Data Corporation

Mario Perone, mperone1@verizon.net tel (781) 587-0086 cell (781) 439-4999

S: Laurel Road E/W: Nason Road/SMS Peds Only Access City, State: Swampscott, MA Client: GEOD/C. Emilius

				Laure	l Road								
				From	South								
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1													
Peak Hour for Entire Intersection Begins at 07:00 AM													
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	3	0	0	3	0	0	0	0	0	0	8	8	11
07:45 AM	1	0	0	1	0	0	0	0	0	0	3	3	4
Total Volume	4	0	0	4	0	0	0	0	0	0	11	11	15
% App. Total	100	0	0		0	0	0		0	0	100		
PHF	.333	.000	.000	.333	.000	.000	.000	.000	.000	.000	.344	.344	.341



S: Laurel Road E/W: Nason Road/SMS Peds Only Access City, State: Swampscott, MA Client: GEOD/C. Emilius

Groups Printed- Cars & Peds													
		Nason Road			Laurel Road		SMS						
		From East			From South								
Start Time	Thru	Left	Peds	Right	Left	Peds	Right	Thru	Peds	Int. Total			
07:00 AM	0	0	0	0	0	0	0	0	0	0			
07:15 AM	0	0	0	0	0	0	0	0	0	0			
07:30 AM	3	0	0	0	0	0	0	0	8	11			
07:45 AM	1	0	0	0	0	0	0	0	3	4			
Total	4	0	0	0	0	0	0	0	11	15			
08:00 AM	0	0	0	0	0	0	0	0	0	0			
08:15 AM	0	0	0	0	0	0	0	0	0	0			
08:30 AM	0	0	0	0	0	0	0	0	0	0			
08:45 AM	0	0	0	0	0	0	0	0	0	0			
Total	0	0	0	0	0	0	0	0	0	0			
Grand Total	4	0	0	0	0	0	0	0	11	15			
Apprch %	100	0	0	0	0	0	0	0	100				
Total %	26.7	0	0	0	0	0	0	0	73.3				

		Nason	Road			Laure	Road						
				From	South								
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1													
Peak Hour for Entire Inter	section Begins	at 07:00 AM											
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	3	0	0	3	0	0	0	0	0	0	8	8	11
07:45 AM	1	0	0	1	0	0	0	0	0	0	3	3	4
Total Volume	4	0	0	4	0	0	0	0	0	0	11	11	15
% App. Total	100	0	0		0	0	0		0	0	100		
PHF	.333	.000	.000	.333	.000	.000	.000	.000	.000	.000	.344	.344	.341
N: SMS Westerly Exit Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name : 04404AA Site Code : 04404 Start Date : 4/1/2014 Page No : 1

	SMS	S Westerly Exit	Only Drives	way	Forest Avenue Forest Avenue								
Start Time	Pight	From N	Orth Pade	App. Total	Pight	From I Thru	Pade	App. Total	Thru	From V	Pede	App. Total	Int Total
Peak Hour Analysis From	02:00 PM to (	03:45 PM - Peak	1 of 1	App. Total	Right	Tinu	Teus	App. Total	Tinu	Lett	Teus	App. Total	Int. Total
Peak Hour for Entire Inter-	section Begins	s at 02:00 PM											
02:00 PM	23	14	1	38	0	12	0	12	58	0	0	58	108
02:15 PM	10	6	56	72	0	22	0	22	42	0	0	42	136
02:30 PM	48	13	47	108	0	22	0	22	51	0	0	51	181
O2:45 PM	/	38	104	230	0	21	0	21	38	0	0	38	/1
% App. Total	38.3	16.5	45.2	250	0	100	0	11	100	0	0	105	490
PHF	.458	.679	.464	.532	.000	.875	.000	.875	.815	.000	.000	.815	.685
Cars & Peds	87	38	101	226	0	74	0	74	187	0	0	187	487
% Cars & Peds	98.9	100	97.1	98.3	0	96.1	0	96.1	98.9	0	0	98.9	98.2
Trucks & Bikes	0	0	20	3	0	0	0	0	0	0	0	0	3
School Buses	1	0	2.9	1.5	0	3	0	3	2	0	0	2	0.0
% School Buses	1.1	0	Ő	0.4	Ő	3.9	Ő	3.9	1.1	Ő	0	1.1	1.2
	Г				SMS Wes	sterly Exit O	nly Drivewa	ay					
						1n 226	226						
					0	3	3						
					0	1	1						
							230						
					l r								
						87 38	101						
						0 0	3						
							0						
					Ria	ht Left	Peds						
					•								
					Peak	k Hour	<sup>-</sup> Data	l					
		6 0 348 554 0 348				<b></b>							
		P	000					<b></b>	꼬	22 1	3 L		
	e			Ľ–e		North					<u>"</u> _		
	ent	87 0 89 89		0	Peak Hour	Begins at 0	2:00 PM	]	-0000		- F		
	Av	트 ~ ~	18					│ ←	3	H.I.	. ⊐ A		
	est				Cars & Peo	1S likoc			<sup>2</sup> 0 <del>2</del> μ	7007	Ver		
	For	<u>-040</u>	000	0 g	School Bus	Ses			P		_ le		
		10 16		Ъ	0011001 244			1	species of		J, Z		
										04 5 0 0			

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Groups Printed- Cars & Peds - Trucks & Bikes - School Buses													
	SMS West	terly Exit Only D	riveway		Forest Avenue			Forest Avenue					
		From North			From East		From West						
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total			
02:00 PM	23	14	1	0	12	0	58	0	0	108			
02:15 PM	10	6	56	0	22	0	42	0	0	136			
02:30 PM	48	13	47	0	22	0	51	0	0	181			
02:45 PM	7	5	0	0	21	0	38	0	0	71			
Total	88	38	104	0	77	0	189	0	0	496			
03:00 PM	11	6	7	0	18	0	31	0	0	73			
03:15 PM	7	1	1	0	11	0	22	0	0	42			
03:30 PM	2	6	2	0	19	0	23	0	0	52			
03:45 PM	5	4	0	0	11	0	19	0	0	39			
Total	25	17	10	0	59	0	95	0	0	206			
Grand Total	113	55	114	0	136	0	284	0	0	702			
Apprch %	40.1	19.5	40.4	0	100	0	100	0	0				
Total %	16.1	7.8	16.2	0	19.4	0	40.5	0	0				
Cars & Peds	112	55	110	0	132	0	282	0	0	691			
% Cars & Peds	99.1	100	96.5	0	97.1	0	99.3	0	0	98.4			
Trucks & Bikes	0	0	4	0	0	0	0	0	0	4			
% Trucks & Bikes	0	0	3.5	0	0	0	0	0	0	0.6			
School Buses	1	0	0	0	4	0	2	0	0	7			
% School Buses	0.9	0	0	0	2.9	0	0.7	0	0	1			

	SMS	SMS Westerly Exit Only Driveway				Forest A	Avenue		Forest Avenue				
		From N	North			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	k 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 02:00 PM											
02:00 PM	23	14	1	38	0	12	0	12	58	0	0	58	108
02:15 PM	10	6	56	72	0	22	0	22	42	0	0	42	136
02:30 PM	48	13	47	108	0	22	0	22	51	0	0	51	181
02:45 PM	7	5	0	12	0	21	0	21	38	0	0	38	71
Total Volume	88	38	104	230	0	77	0	77	189	0	0	189	496
% App. Total	38.3	16.5	45.2		0	100	0		100	0	0		
PHF	.458	.679	.464	.532	.000	.875	.000	.875	.815	.000	.000	.815	.685
Cars & Peds	87	38	101	226	0	74	0	74	187	0	0	187	487
% Cars & Peds	98.9	100	97.1	98.3	0	96.1	0	96.1	98.9	0	0	98.9	98.2
Trucks & Bikes	0	0	3	3	0	0	0	0	0	0	0	0	3
% Trucks & Bikes	0	0	2.9	1.3	0	0	0	0	0	0	0	0	0.6
School Buses	1	0	0	1	0	3	0	3	2	0	0	2	6
% School Buses	1.1	0	0	0.4	0	3.9	0	3.9	1.1	0	0	1.1	1.2

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Groups Printed- Cars & Peds													
	SMS West	erly Exit Only Di	riveway		Forest Avenue								
		From North			From East								
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total			
02:00 PM	22	14	1	0	12	0	58	0	0	107			
02:15 PM	10	6	53	0	22	0	42	0	0	133			
02:30 PM	48	13	47	0	20	0	49	0	0	177			
02:45 PM	7	5	0	0	20	0	38	0	0	70			
Total	87	38	101	0	74	0	187	0	0	487			
03:00 PM	11	6	6	0	18	0	31	0	0	72			
03:15 PM	7	1	1	0	10	0	22	0	0	41			
03:30 PM	2	6	2	0	19	0	23	0	0	52			
03:45 PM	5	4	0	0	11	0	19	0	0	39			
Total	25	17	9	0	58	0	95	0	0	204			
Grand Total	112	55	110	0	132	0	282	0	0	691			
Apprch %	40.4	19.9	39.7	0	100	0	100	0	0				
Total %	16.2	8	15.9	0	19.1	0	40.8	0	0				

	SMS	Westerly Exit	t Only Drive	eway		Forest	Avenue		Forest Avenue				
		From 1	North			From East From West							
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	3:45 PM - Pea	ık 1 of 1										
Peak Hour for Entire Inter-	section Begins a	at 02:00 PM											
02:00 PM	22	14	1	37	0	12	0	12	58	0	0	58	107
02:15 PM	10	6	53	69	0	22	0	22	42	0	0	42	133
02:30 PM	48	13	47	108	0	20	0	20	49	0	0	49	177
02:45 PM	7	5	0	12	0	20	0	20	38	0	0	38	70
Total Volume	87	38	101	226	0	74	0	74	187	0	0	187	487
% App. Total	38.5	16.8	44.7		0	100	0		100	0	0		
PHF	.453	.679	.476	.523	.000	.841	.000	.841	.806	.000	.000	.806	.688

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Groups Printed- Trucks & Bikes													
	SMS Weste	erly Exit Only Dr	iveway		Forest Avenue								
		From North			From East								
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total			
02:00 PM	0	0	0	0	0	0	0	0	0	0			
02:15 PM	0	0	3	0	0	0	0	0	0	3			
02:30 PM	0	0	0	0	0	0	0	0	0	0			
02:45 PM	0	0	0	0	0	0	0	0	0	0			
Total	0	0	3	0	0	0	0	0	0	3			
03:00 PM	0	0	1	0	0	0	0	0	0	1			
03:15 PM	0	0	0	0	0	0	0	0	0	0			
03:30 PM	0	0	0	0	0	0	0	0	0	0			
03:45 PM	0	0	0	0	0	0	0	0	0	0			
Total	0	0	1	0	0	0	0	0	0	1			
Grand Total	0	0	4	0	0	0	0	0	0	4			
Apprch %	0	0	100	0	0	0	0	0	0				
Total %	0	0	100	0	0	0	0	0	0				

	SMS	SMS Westerly Exit Only Driveway				Forest Avenue				Forest Avenue			
		From 1	North		From East From We					n West			
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	ık 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 02:15 PM											
02:15 PM	0	0	3	3	0	0	0	0	0	0	0	0	3
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	1	1	0	0	0	0	0	0	0	0	1
Total Volume	0	0	4	4	0	0	0	0	0	0	0	0	4
% App. Total	0	0	100		0	0	0		0	0	0		
PHF	.000	.000	.333	.333	.000	.000	.000	.000	.000	.000	.000	.000	.333

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Groups Printed- School Buses													
	SMS Weste	erly Exit Only Dr	iveway		Forest Avenue								
		From North			From East		From West						
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total			
02:00 PM	1	0	0	0	0	0	0	0	0	1			
02:15 PM	0	0	0	0	0	0	0	0	0	0			
02:30 PM	0	0	0	0	2	0	2	0	0	4			
02:45 PM	0	0	0	0	1	0	0	0	0	1			
Total	1	0	0	0	3	0	2	0	0	6			
03:00 PM	0	0	0	0	0	0	0	0	0	0			
03:15 PM	0	0	0	0	1	0	0	0	0	1			
03:30 PM	0	0	0	0	0	0	0	0	0	0			
03:45 PM	0	0	0	0	0	0	0	0	0	0			
Total	0	0	0	0	1	0	0	0	0	1			
Grand Total	1	0	0	0	4	0	2	0	0	7			
Apprch %	100	0	0	0	100	0	100	0	0				
Total %	14.3	0	0	0	57.1	0	28.6	0	0				

	SMS	Westerly Exit	t Only Drive	eway		Forest	Avenue						
		From 1	North			Fron	n East			From	n West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	3:45 PM - Pea	ık 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 02:00 PM											
02:00 PM	1	0	0	1	0	0	0	0	0	0	0	0	1
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	2	0	2	2	0	0	2	4
02:45 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	1	0	0	1	0	3	0	3	2	0	0	2	6
% App. Total	100	0	0		0	100	0		100	0	0		
PHF	.250	.000	.000	.250	.000	.375	.000	.375	.250	.000	.000	.250	.375

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

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	SMS	SMS Middle Enter Only Driveway Forest Avenue Forest Avenue											
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds A	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From (	02:00 PM to 03	3:45 PM - Peak	1 of 1										
Peak Hour for Entire Inters	section Begins	at 02:00 PM		. 1								1	
02:00 PM 02:15 PM	1	0	1 105	105	11	13	0	24	54	16 26	0 25	70	96 217
02:30 PM	0	0	71	71	14	23 22	1	39	43	20 22	23 27	92	197
02:45 PM	0	0	0	0	3	20	1	24	28	13	0	41	65
Total Volume	1	0	177	178	39	78	4	121	147	77	52	276	575
% App. Total PHF	250	0	421	124	<u> </u>	<u>64.5</u> 848	<u> </u>	776	<u> </u>	740	18.8	750	662
Cars & Peds	1	0	174	175	39	75	4	118	145	77	52	274	567
% Cars & Peds	100	0	98.3	98.3	100	96.2	100	97.5	98.6	100	100	99.3	98.6
Trucks & Bikes	0	0	3	3	0	0	0	0	0	0	0	0	3
School Buses	0	0	0	0	0	3	0	3	2	0	0	2	0.5
% School Buses	0	0	0	0	0	3.8	0	2.5	1.4	0	0	0.7	0.9
					SMS Mid	dle Enter Or	nly Driveway						
					Out	In 175	l otal						
					0	3	3						
					0	0	0						
					116		294						
						1 0	174						
						0 0	0						
						1 0	177						
					Rig	ht Left	Peds						
					←	<b>└→</b>							
					Peak	k Hour	Data						
		350		-		<b></b>							
	Ĥ			≒_ <b>↑</b>				<b>≜</b>	<u></u>		ut		
	enc			Ľ	<b>D</b>	North			<sup>11</sup> 3003		- For		
	Ver	274	20042	15 /	Peak Hour	Begins at 0	2:00 PM	. :			est		
	st ⊳			- Ē▶	Cars & Peo	ds				121 0 0			
	ore		2002	2 2	Trucks & B	ikes		-	0		nu		
	۳.		4	Ped	School Bus	ses		0			, H <sup>®</sup>		
				]_					4004	268 5 0	Stal		
											-		

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Groups Printed- Cars & Peds - Trucks & Bikes - School Buses													
	SMS Midd	le Enter Only Dr	iveway		Forest Avenue			Forest Avenue					
		From North			From East			From West					
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total			
02:00 PM	1	0	1	11	13	0	54	16	0	96			
02:15 PM	0	0	105	14	23	2	22	26	25	217			
02:30 PM	0	0	71	11	22	1	43	22	27	197			
02:45 PM	0	0	0	3	20	1	28	13	0	65			
Total	1	0	177	39	78	4	147	77	52	575			
03:00 PM	0	0	9	4	19	0	30	6	0	68			
03:15 PM	0	0	1	0	10	0	19	3	0	33			
03:30 PM	0	0	3	3	19	1	27	2	1	56			
03:45 PM	0	0	0	3	12	0	17	6	0	38			
Total	0	0	13	10	60	1	93	17	1	195			
Grand Total	1	0	190	49	138	5	240	94	53	770			
Apprch %	0.5	0	99.5	25.5	71.9	2.6	62	24.3	13.7				
Total %	0.1	0	24.7	6.4	17.9	0.6	31.2	12.2	6.9				
Cars & Peds	1	0	186	49	134	5	238	94	53	760			
% Cars & Peds	100	0	97.9	100	97.1	100	99.2	100	100	98.7			
Trucks & Bikes	0	0	4	0	0	0	0	0	0	4			
% Trucks & Bikes	0	0	2.1	0	0	0	0	0	0	0.5			
School Buses	0	0	0	0	4	0	2	0	0	6			
% School Buses	0	0	0	0	2.9	0	0.8	0	0	0.8			

	SMS	Middle Enter	Only Drive	way	Forest Avenue					Forest A	venue		
		From N	North			From	n East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	k 1 of 1										
Peak Hour for Entire Inters	ection Begins a	at 02:00 PM											
02:00 PM	1	0	1	2	11	13	0	24	54	16	0	70	96
02:15 PM	0	0	105	105	14	23	2	39	22	26	25	73	217
02:30 PM	0	0	71	71	11	22	1	34	43	22	27	92	197
02:45 PM	0	0	0	0	3	20	1	24	28	13	0	41	65
Total Volume	1	0	177	178	39	78	4	121	147	77	52	276	575
% App. Total	0.6	0	99.4		32.2	64.5	3.3		53.3	27.9	18.8		
PHF	.250	.000	.421	.424	.696	.848	.500	.776	.681	.740	.481	.750	.662
Cars & Peds	1	0	174	175	39	75	4	118	145	77	52	274	567
% Cars & Peds	100	0	98.3	98.3	100	96.2	100	97.5	98.6	100	100	99.3	98.6
Trucks & Bikes	0	0	3	3	0	0	0	0	0	0	0	0	3
% Trucks & Bikes	0	0	1.7	1.7	0	0	0	0	0	0	0	0	0.5
School Buses	0	0	0	0	0	3	0	3	2	0	0	2	5
% School Buses	0	0	0	0	0	3.8	0	2.5	1.4	0	0	0.7	0.9

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				Groups Print	ed- Cars & Peds					
	SMS Mide	dle Enter Only Di	riveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
02:00 PM	1	0	1	11	13	0	54	16	0	96
02:15 PM	0	0	102	14	23	2	22	26	25	214
02:30 PM	0	0	71	11	19	1	41	22	27	192
02:45 PM	0	0	0	3	20	1	28	13	0	65
Total	1	0	174	39	75	4	145	77	52	567
03:00 PM	0	0	8	4	19	0	30	6	0	67
03:15 PM	0	0	1	0	9	0	19	3	0	32
03:30 PM	0	0	3	3	19	1	27	2	1	56
03:45 PM	0	0	0	3	12	0	17	6	0	38
Total	0	0	12	10	59	1	93	17	1	193
Grand Total	1	0	186	49	134	5	238	94	53	760
Apprch %	0.5	0	99.5	26.1	71.3	2.7	61.8	24.4	13.8	
Total %	0.1	0	24.5	6.4	17.6	0.7	31.3	12.4	7	

	SMS	Middle Enter	Only Drive	way		Forest A	Avenue						
		From N	Jorth			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	k 1 of 1										
Peak Hour for Entire Inter-	section Begins a	t 02:00 PM											
02:00 PM	1	0	1	2	11	13	0	24	54	16	0	70	96
02:15 PM	0	0	102	102	14	23	2	39	22	26	25	73	214
02:30 PM	0	0	71	71	11	19	1	31	41	22	27	90	192
02:45 PM	0	0	0	0	3	20	1	24	28	13	0	41	65
Total Volume	1	0	174	175	39	75	4	118	145	77	52	274	567
% App. Total	0.6	0	99.4		33.1	63.6	3.4		52.9	28.1	19		
PHF	.250	.000	.426	.429	.696	.815	.500	.756	.671	.740	.481	.761	.662

N: SMS Middle Enter Only Driveway E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

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				Groups Printe	d- Trucks & Bike	s				
	SMS Mide	dle Enter Only Dr	iveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	3	0	0	0	0	0	0	3
02:30 PM	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0
Total	0	0	3	0	0	0	0	0	0	3
03:00 PM	0	0	1	0	0	0	0	0	0	1
03:15 PM	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	0	0	0	0	0	0	0
Total	0	0	1	0	0	0	0	0	0	1
Grand Total	0	0	4	0	0	0	0	0	0	4
Apprch %	0	0	100	0	0	0	0	0	0	
Total %	0	0	100	0	0	0	0	0	0	

	SMS	Middle Enter	Only Drive	eway		Forest A	Venue						
		From N	North			From	East			From	West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	k 1 of 1										
Peak Hour for Entire Inter-	section Begins a	at 02:15 PM											
02:15 PM	0	0	3	3	0	0	0	0	0	0	0	0	3
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	1	1	0	0	0	0	0	0	0	0	1
Total Volume	0	0	4	4	0	0	0	0	0	0	0	0	4
% App. Total	0	0	100		0	0	0		0	0	0		
PHF	.000	.000	.333	.333	.000	.000	.000	.000	.000	.000	.000	.000	.333

File Name	: 04404BB
Site Code	: 04404
Start Date	: 4/1/2014
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				Groups Printe	d- School Buses					
	SMS Mide	ile Enter Only Dr	iveway		Forest Avenue			Forest Avenue		
		From North			From East			From West		
Start Time	Right	Left	Peds	Right	Thru	Peds	Thru	Left	Peds	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	3	0	2	0	0	5
02:45 PM	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	3	0	2	0	0	5
03:00 PM	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	0	1	0	0	0	0	1
03:30 PM	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	1	0	0	0	0	1
Grand Total	0	0	0	0	4	0	2	0	0	6
Apprch %	0	0	0	0	100	0	100	0	0	
Total %	0	0	0	0	66.7	0	33.3	0	0	

	SMS	Middle Enter	Only Drive	eway		Forest	Avenue						
		From 1	North			Fron	n East			From	n West		
Start Time	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	3:45 PM - Pea	ık 1 of 1										
Peak Hour for Entire Inters	section Begins a	at 02:30 PM											
02:30 PM	0	0	0	0	0	3	0	3	2	0	0	2	5
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	1
Total Volume	0	0	0	0	0	4	0	4	2	0	0	2	6
% App. Total	0	0	0		0	100	0		100	0	0		
PHF	.000	.000	.000	.000	.000	.333	.000	.333	.250	.000	.000	.250	.300

N/S: SMS East Drive/Sargent Road E/W: Forest Avenue City, State: Swampscott, MA Client: GEOD/C. Emilius

F

File Name : 04404CC Site Code : 04404 Start Date : 4/1/2014 Page No : 1

$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	5         0         54           1         35         55           1         35         55           1         47         90           7         0         30           14         82         229           6.1         35.8         550	Int. Total 100 201 225 63 589 
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	$   \begin{array}{r}     100 \\     201 \\     225 \\     \underline{63} \\     589 \\   \end{array} $
$ \frac{1}{2} \text{Hour for Entre Intersection Begins at 0.200 PM}{20200 PM} = 1 \\ 02:15 PM = 1 \\ 0 \\ 02:15 PM = 1 \\ 0 \\ 02:5 PM = 1 \\ 0 \\ 0 \\ 13 \\ 0 \\ 13 \\ 0 \\ 11 \\ 0 \\ 13 \\ 0 \\ 11 \\ 0 \\ 12 \\ 10 \\ 10 \\ 10 \\ 10 \\$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	100 201 <b>225</b> <u>63</u> 589
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	100 201 <b>225</b> <u>63</u> 589
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	201 225 63 589
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	225 63 589
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	7         0         30           14         82         229           6.1         35.8           500         436         636	<u>63</u> 589
10air       00air       13       1       8       140       13       00       58       0       103       23       3       4       47       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       117       47       88       118       140       5       59       38       0       102       22       3       42       47       114       46       85       1       117       66       100       100       100       100       100       99.0       95.7       100       95.5       100       97.4       97.9       98.8       10         Trucks 8 ikes       0	14         82         229           6.1         35.8	
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	500 436 636	
$\begin{array}{c} \hline \text{Cars \& Peds} & 13 & 1 & 8 & 118 & 140 & 5 & 59 & 38 & 0 & 102 & 22 & 3 & 42 & 47 & 114 & 46 & 85 & 1 \\ \hline \text{w Cars & Peds} & 100 & 100 & 100 & 100 & 100 & 98.3 & 100 & 0 & 95.7 & 100 & 95.5 & 100 & 97.4 & 97.9 & 98.8 & 10 \\ \hline \text{Tracks & Bikes} & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & $	.500 .450 .050	.654
% Cars & Peds       100       100       100       100       100       100       98.3       100       0       99.0       95.7       100       95.5       100       97.4       97.9       98.8       10         Trucks & Bikes       0	14 82 227	583
Trucks & Bikes       0	100 100 99.1	99.0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	0 0 0	0
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		0
SMAXIMAN ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) ( ) (	0 0 2 0 2	10
SMS Easterly Driveway         Out       In       Total         22       140       162         0       0       0         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         13       1       8         14       10       10         13       1       8         14       10       118         Right       Thru       Left         Peak Hour Data       10	0 0 0.9	1.0
Image: space of the space o		
86         117         203           Out         In         Total           Sargent Road		

File Name	:04404CC
Site Code	: 04404
Start Date	: 4/1/2014
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	Groups Printed- Cars & Peds - Trucks & Bikes - School Buses																
	SM	S Easterly l	Driveway			Forest Av	enue			Sargent F	Road			Forest Av	enue		
		From No	orth			From E	ast			From So	outh			From W	est		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
02:00 PM	1	0	1	0	1	14	6	0	11	0	12	0	8	41	5	0	100
02:15 PM	2	0	2	56	0	23	18	0	4	2	14	25	10	9	1	35	201
02:30 PM	3	0	1	62	1	14	14	0	6	0	12	22	23	19	1	47	225
02:45 PM	7	1	4	0	3	9	0	0	2	1	6	0	6	17	7	0	63
Total	13	1	8	118	5	60	38	0	23	3	44	47	47	86	14	82	589
03:00 PM	5	6	3	3	0	12	1	0	1	2	5	3	9	19	3	5	77
03:15 PM	2	2	1	0	0	4	4	0	4	0	3	0	4	12	4	0	40
03:30 PM	1	1	2	0	1	18	3	0	8	0	3	0	9	16	1	0	63
03:45 PM	1	0	0	0	0	12	3	0	2	0	2	0	7	9	1	0	37
Total	9	9	6	3	1	46	11	0	15	2	13	3	29	56	9	5	217
Grand Total	22	10	14	121	6	106	49	0	38	5	57	50	76	142	23	87	806
Apprch %	13.2	6	8.4	72.5	3.7	65.8	30.4	0	25.3	3.3	38	33.3	23.2	43.3	7	26.5	
Total %	2.7	1.2	1.7	15	0.7	13.2	6.1	0	4.7	0.6	7.1	6.2	9.4	17.6	2.9	10.8	
Cars & Peds	22	10	14	121	6	105	48	0	37	5	54	50	75	141	23	87	798
% Cars & Peds	100	100	100	100	100	99.1	98	0	97.4	100	94.7	100	98.7	99.3	100	100	99
Trucks & Bikes	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
% Trucks & Bikes	0	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0.1
School Buses	0	0	0	0	0	1	0	0	1	0	3	0	1	1	0	0	7
% School Buses	0	0	0	0	0	0.9	0	0	2.6	0	5.3	0	1.3	0.7	0	0	0.9

		SMS Ea	IS Easterly Driveway Forest Avenue								Sargent Road Forest Avenue										
		F	rom Nort	h			1	From Eas	t			F	<u>rom Sou</u>	th			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	is From 0	2:00 PM	to 03:45	PM - Pe	ak 1 of 1																
Peak Hour for Ent	ire Interse	ection Beg	gins at 02	2:00 PM																	
02:00 PM	1	0	1	0	2	1	14	6	0	21	11	0	12	0	23	8	41	5	0	54	100
02:15 PM	2	0	2	56	60	0	23	18	0	41	4	2	14	25	45	10	9	1	35	55	201
02:30 PM	3	0	1	62	66	1	14	14	0	29	6	0	12	22	40	23	19	1	47	90	225
02:45 PM	7	1	4	0	12	3	9	0	0	12	2	1	6	0	9	6	17	7	0	30	63
Total Volume	13	1	8	118	140	5	60	38	0	103	23	3	44	47	117	47	86	14	82	229	589
% App. Total	9.3	0.7	5.7	84.3		4.9	58.3	36.9	0		19.7	2.6	37.6	40.2		20.5	37.6	6.1	35.8		
PHF	.464	.250	.500	.476	.530	.417	.652	.528	.000	.628	.523	.375	.786	.470	.650	.511	.524	.500	.436	.636	.654
Cars & Peds	13	1	8	118	140	5	59	38	0	102	22	3	42	47	114	46	85	14	82	227	583
% Cars & Peds	100	100	100	100	100	100	98.3	100	0	99.0	95.7	100	95.5	100	97.4	97.9	98.8	100	100	99.1	99.0
Trucks & Bikes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Trucks & Bikes	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
School Buses	0	0	0	0	0	0	1	0	0	1	1	0	2	0	3	1	1	0	0	2	6
% School Buses	0	0	0	0	0	0	1.7	0	0	1.0	4.3	0	4.5	0	2.6	2.1	1.2	0	0	0.9	1.0

File Name	: 04404CC
Site Code	: 04404
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							Groups P	rinted- Ca	rs & Peds								
	SM	S Easterly	Driveway			Forest A	venue			Sargent F	Road			Forest A	venue		
		From N	orth			From I	East			From Se	outh			From V	West		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
02:00 PM	1	0	1	0	1	14	6	0	11	0	12	0	8	41	5	0	100
02:15 PM	2	0	2	56	0	23	18	0	3	2	13	25	10	9	1	35	199
02:30 PM	3	0	1	62	1	13	14	0	6	0	11	22	22	18	1	47	221
02:45 PM	7	1	4	0	3	9	0	0	2	1	6	0	6	17	7	0	63
Total	13	1	8	118	5	59	38	0	22	3	42	47	46	85	14	82	583
03:00 PM	5	6	3	3	0	12	1	0	1	2	4	3	9	19	3	5	76
03:15 PM	2	2	1	0	0	4	4	0	4	0	3	0	4	12	4	0	40
03:30 PM	1	1	2	0	1	18	2	0	8	0	3	0	9	16	1	0	62
03:45 PM	1	0	0	0	0	12	3	0	2	0	2	0	7	9	1	0	37
Total	9	9	6	3	1	46	10	0	15	2	12	3	29	56	9	5	215
Grand Total	22	10	14	121	6	105	48	0	37	5	54	50	75	141	23	87	798
Apprch %	13.2	6	8.4	72.5	3.8	66	30.2	0	25.3	3.4	37	34.2	23	43.3	7.1	26.7	
Total %	2.8	1.3	1.8	15.2	0.8	13.2	6	0	4.6	0.6	6.8	6.3	9.4	17.7	2.9	10.9	

		SMS Ea	sterly D	riveway			For	est Aver	nue			Sa	rgent Ro	ad							
		F	rom Nor	th			F	From Eas	st			F	rom Sou	th			F	rom We	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	sis From (	2:00 PM	to 03:45	PM - Pe	ak 1 of 1																
Peak Hour for En	tire Inters	ection Be	gins at 02	2:00 PM																	
02:00 PM	1	0	1	0	2	1	14	6	0	21	11	0	12	0	23	8	41	5	0	54	100
02:15 PM	2	0	2	56	60	0	23	18	0	41	3	2	13	25	43	10	9	1	35	55	199
02:30 PM	3	0	1	62	66	1	13	14	0	28	6	0	11	22	39	22	18	1	47	88	221
02:45 PM	7	1	4	0	12	3	9	0	0	12	2	1	6	0	9	6	17	7	0	30	63
Total Volume	13	1	8	118	140	5	59	38	0	102	22	3	42	47	114	46	85	14	82	227	583
% App. Total	9.3	0.7	5.7	84.3		4.9	57.8	37.3	0		19.3	2.6	36.8	41.2		20.3	37.4	6.2	36.1		
PHF	.464	.250	.500	.476	.530	.417	.641	.528	.000	.622	.500	.375	.808	.470	.663	.523	.518	.500	.436	.645	.660

File Name	:04404CC
Site Code	: 04404
Start Date	: 4/1/2014
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							Groups Pri	nted- Truc	ks & Bikes								
	SM	S Easterly	Driveway			Forest Av	enue			Sargent R	Road			Forest Av	/enue		
		From No	orth			From E	last			From Sc	outh			From W	Vest		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
Grand Total	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
Apprch %	0	0	0	0	0	0	100	0	0	0	0	0	0	0	0	0	
Total %	0	0	0	0	0	0	100	0	0	0	0	0	0	0	0	0	

		SMS Ea	sterly D	riveway			Forest Avenue					Sargent Road					Forest Avenue					
		F	rom Nor	th			From East					F	rom Sou	th			F	rom We	st			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total	
Peak Hour Analys	sis From (	02:00 PM	to 03:45	PM - Pe	ak 1 of 1																	
Peak Hour for Ent	tire Inters	ection Be	gins at 0	2:45 PM																		
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:30 PM	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1	
Total Volume	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1	
% App. Total	0	0	0	0		0	0	100	0		0	0	0	0		0	0	0	0			
PHF	.000	.000	.000	.000	.000	.000	.000	.250	.000	.250	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.250	

File Name	: 04404CC
Site Code	: 04404
Start Date	: 4/1/2014
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							Groups Pi	rinted- Sch	ool Buses								
	SM	S Easterly	Driveway			Forest Av	/enue			Sargent l	Road		Forest Avenue				
		From N	orth			From E	East			From S	outh			From W	/est		
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	2
02:30 PM	0	0	0	0	0	1	0	0	0	0	1	0	1	1	0	0	4
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	1	0	0	1	0	2	0	1	1	0	0	6
03:00 PM	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
03:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
Grand Total	0	0	0	0	0	1	0	0	1	0	3	0	1	1	0	0	7
Apprch %	0	0	0	0	0	100	0	0	25	0	75	0	50	50	0	0	
Total %	0	0	0	0	0	14.3	0	0	14.3	0	42.9	0	14.3	14.3	0	0	

		SMS Ea	sterly D	riveway			For	est Avei	nue			Sa	rgent Ro	ad			For	est Aver	nue		
		F	rom Nor	th			F	From Eas	st			F	rom Sou	th			F	rom Wes	st		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour Analys	sis From (	02:00 PM	to 03:45	PM - Pe	ak 1 of 1																
Peak Hour for En	tire Inters	ection Be	gins at 0	2:15 PM																	
02:15 PM	0	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	0	0	0	0	2
02:30 PM	0	0	0	0	0	0	1	0	0	1	0	0	1	0	1	1	1	0	0	2	4
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	1
Total Volume	0	0	0	0	0	0	1	0	0	1	1	0	3	0	4	1	1	0	0	2	7
% App. Total	0	0	0	0		0	100	0	0		25	0	75	0		50	50	0	0		
PHF	.000	.000	.000	.000	.000	.000	.250	.000	.000	.250	.250	.000	.750	.000	.500	.250	.250	.000	.000	.250	.438

## Transportation Data Corporation

Mario Perone, mperone1@verizon.net tel (781) 587-0086 cell (781) 439-4999

S: Laurel Road E/W: Nason Road/SMS Peds Only Access City, State: Swampscott, MA Client: GEOD/C. Emilius File Name : 04404DD Site Code : 04404 Start Date : 4/1/2014 Page No : 1

		Nason	Road			Laure	Road			SMS Rear P	eds Exiting		
		From	East			From	South			From	West		
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03	:45 PM - Pea	ak 1 of 1										
Peak Hour for Entire Inter-	section Begins a	at 02:15 PM											
02:15 PM	16	0	0	16	0	0	0	0	0	0	9	9	25
02:30 PM	8	0	0	8	0	0	0	0	0	0	14	14	22
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	1	0	0	1	0	0	0	0	0	0	2	2	3
Total Volume	25	0	0	25	0	0	0	0	0	0	25	25	50
% App. Total	100	0	0		0	0	0		0	0	100		
PHF	.391	.000	.000	.391	.000	.000	.000	.000	.000	.000	.446	.446	.500



S: Laurel Road E/W: Nason Road/SMS Peds Only Access City, State: Swampscott, MA Client: GEOD/C. Emilius

File Name	:04404DD
Site Code	: 04404
Start Date	: 4/1/2014
Page No	:1

Peds	
Peds	
Peds	
	Int. Total
0	0
9	25
14	22
0	0
23	47
2	3
1	1
2	2
0	0
5	6
28	53
100	
52.8	
_	0 23 2 1 2 0 5 5 28 100 52.8

		Nason I	Road			Laure	l Road			SMS Rear I	Peds Exiting		
		From	East			From	South			From	n West		
Start Time	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total	Int. Total
Peak Hour Analysis From	02:00 PM to 03:	45 PM - Pea	k 1 of 1										
Peak Hour for Entire Inter-	section Begins a	t 02:15 PM											
02:15 PM	16	0	0	16	0	0	0	0	0	0	9	9	25
02:30 PM	8	0	0	8	0	0	0	0	0	0	14	14	22
02:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00 PM	1	0	0	1	0	0	0	0	0	0	2	2	3
Total Volume	25	0	0	25	0	0	0	0	0	0	25	25	50
% App. Total	100	0	0		0	0	0		0	0	100		
PHF	.391	.000	.000	.391	.000	.000	.000	.000	.000	.000	.446	.446	.500

# **APPENDIX B**

MIDDLE SCHOOL OPERATIONAL ANALYSIS CALCULATIONS



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	-	$\mathbf{x}$	×	ť	6	*
Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	9	182	246	19	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.990		0.973	
Flt Protected		0.997			0.962	
Satd. Flow (prot)	0	1894	1814	0	1778	0
Flt Permitted		0.997			0.962	
Satd. Flow (perm)	0	1894	1814	0	1778	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		395	105		180	
Travel Time (s)		26.9	7.2		12.3	
Confl. Peds. (#/hr)	80				51	
Peak Hour Factor	0.45	0.68	0.67	0.63	0.25	0.50
Heavy Vehicles (%)	0%	0%	4%	0%	0%	0%
Adj. Flow (vph)	20	268	367	30	32	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	288	397	0	40	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 26.9% Analysis Period (min) 15

ICU Level of Service A

## ۰. ٭ 5 ~ ← EBL EBT WBT SBL Lane Group WBR SBR ø2 **↑**↑ Lane Configurations Volume (vph) 0 271 0 0 0 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 1.00 1.00 1.00 Frt Flt Protected Satd. Flow (prot) 3610 0 0 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 0 3610 0 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) Link Speed (mph) 10 10 10 Link Distance (ft) 157 118 112 Travel Time (s) 10.7 8.0 7.6 Peak Hour Factor 0.92 0.92 0.92 0.92 0.45 0.92 Heavy Vehicles (%) 0% 0% 0% 0% 0% 0% Adj. Flow (vph) 602 0 0 0 0 0 Shared Lane Traffic (%) 0 0 Lane Group Flow (vph) 602 0 0 0 Enter Blocked Intersection No No No No No No Lane Alignment Left Left Left Right Left Right Median Width(ft) 0 0 0 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 NA Turn Type Protected Phases 2 4 Permitted Phases Minimum Split (s) 3.0 3.5 Total Split (s) 3.0 3.5 Total Split (%) 46.2% 54% Maximum Green (s) 1.0 1.0 Yellow Time (s) 2.0 2.0 All-Red Time (s) 0.0 0.5 Lost Time Adjust (s) 0.0 Total Lost Time (s) 2.0 Lead/Lag Lead-Lag Optimize? Act Effct Green (s) 1.0 Actuated g/C Ratio 0.15 v/c Ratio 1.08 Control Delay 60.2 Queue Delay 0.0 Total Delay 60.2 LOS Ε 60.2 Approach Delay

Shaw School 4/9/2014 Baseline

Ε

Approach LOS

	≯	-	-	*	1	<		
ane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø2	
Stops (vph)		115						
Fuel Used(gal)		4						
CO Emissions (g/hr)		271						
NOx Emissions (g/hr)		53						
VOC Emissions (g/hr)		63						
Dilemma Vehicles (#)		0						
Queue Length 50th (ft)		0						
Queue Length 95th (ft)		0						
Internal Link Dist (ft)		77	38		32			
Turn Bay Length (ft)								
Base Capacity (vph)		555						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						
Reduced v/c Ratio		1.08						
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced t	to phase 2:F	Hold and	6:, Start	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 1.08								
Intersection Signal Delay: 60	0.2			In	tersection	LOS: E		
Intersection Capacity Utiliza	tion 10.8%			IC	U Level o	of Service	ł	
Analysis Period (min) 15								
Splits and Phases: 2:								

ø2 (R)		<b>→</b> ø4	
3.5 s		3 s	

	-	*	٦	1	$\searrow$	$\rightarrow$	
Lane Group	WBL	WBR	NBL	NBR	SEL	SER	
Lane Configurations	¥		Y		Y		
Volume (vph)	19	156	109	31	131	59	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.880		0.980		0.949		
Flt Protected	0.994		0.959		0.970		
Satd. Flow (prot)	1662	0	1699	0	1749	0	
Flt Permitted	0.994		0.959		0.970		
Satd. Flow (perm)	1662	0	1699	0	1749	0	
Link Speed (mph)	10		10		10		
Link Distance (ft)	408		65		105		
Travel Time (s)	27.8		4.4		7.2		
Confl. Peds. (#/hr)	28	2	28			28	
Peak Hour Factor	0.59	0.63	0.39	0.65	0.68	0.51	
Heavy Vehicles (%)	0%	0%	6%	0%	0%	0%	
Adj. Flow (vph)	32	248	279	48	193	116	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	280	0	327	0	309	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12		12		12		
Link Offset(ft)	0		0		0		
Crosswalk Width(ft)	16		16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15	9	15	9	
Sign Control	Free		Stop		Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 41.6%			IC	CU Level of	of Service A	A

Analysis Period (min) 15

	-	•	<b>†</b>	1	1	Ŧ			
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	ø8		
Lane Configurations			*			*			
Volume (vph)	0	0	140	0	0	76			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Frt									
Flt Protected									
Satd Flow (prot)	0	0	1827	0	0	1900			
-It Permitted		Ŭ		0	Ű				
Satd Flow (perm)	0	0	1827	0	0	1900			
Right Turn on Red	Ū	Yes	1027	Yes	Ű	1700			
Satd Flow (RTOR)		105		100					
ink Speed (mph)	10		10			10			
ink Distance (ft)	228		261			65			
Fravel Time (s)	15 5		17.8			4 4			
Peak Hour Factor	0.92	0.92	0.39	0.92	0.92	0.51			
Heavy Vehicles (%)	0.72	0%	4%	0%	0%	0%			
Adi Flow (vnh)	070	0,0	250	070	070	149			
Shared Lane Traffic (%)	U	U	337	U	U	177			
ane Group Flow (vph)	Ο	Ο	350	0	0	140			
-and Group Flow (vpn) -nter Blocked Intersection	No	No	No	No	No	No			
and Alianment	L oft	Pight	Loft	Pight	Loft	Loft			
Jodian Width(ft)		Night	0	Right	Len				
ink Offsot(ft)	0		0			0			
Crosswalk Width(ft)	16		16			16			
Two way Loft Turn Lano	10		10			10			
Hoadway Eactor	1 00	1 00	1 00	1 00	1 00	1 00			
Turning Spood (mph)	1.00	1.00	1.00	1.00	1.00	1.00			
Turn Typo	15	9	NIA	9	15	NIΛ			
Protoctod Dhasas			2			INA 6	0		
Protected Phases			Z			0	0		
Ainimum Split (c)			2.0			2.0	2 5		
(5) Total Split (5)			3.U 2.O			3.U 2.0	3.0 2.5		
Total Split (S)			3.0			3.U 16 00/	5.5		
Javimum Croon (c)			40.Z%			40.2%	0470 1 0		
Vallow Time (s)			1.0			1.0	1.0		
			2.0			2.0	2.0		
ast Time Adjust (s)			0.0			0.0	0.5		
Lust Time Aujust (S)			0.0			0.0			
			2.0			2.0			
_eau/Lay									
Leau-Lay Optimize?			1.0			1.0			
ALL EIICI GIEEN (S)			1.0			1.0			
Actualeu y/C Kallo			0.15			0.15			
//L Kallo			1.20			0.51			
Jonitol Delay			105.0			6.8			
Lueue Delay			0.0			0.0			
lotal Delay			165.6			6.8			
_US						A			
Approach Delay			165.6			6.8			
Approach LOS			F			A			

Shaw School 4/9/2014 Baseline

	-	•	1	1	1	Ŧ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Stops (vph)			157			7	
Fuel Used(gal)			5			0	
CO Emissions (g/hr)			366			12	
NOx Emissions (g/hr)			71			2	
VOC Emissions (g/hr)			85			3	
Dilemma Vehicles (#)			0			0	
Queue Length 50th (ft)			0			0	
Queue Length 95th (ft)			0			m0	
Internal Link Dist (ft)	148		181			1	
Turn Bay Length (ft)							
Base Capacity (vph)			281			292	
Starvation Cap Reductn			0			0	
Spillback Cap Reductn			0			0	
Storage Cap Reductn			0			0	
Reduced v/c Ratio			1.28			0.51	
Intersection Summary							
Area Type: (	Other						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	o phase 2:	NBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 1.28							
Intersection Signal Delay: 11	9.0			In	tersectior	n LOS: F	
Intersection Capacity Utilizat	ion 10.7%			IC	U Level o	of Service A	۱.
Analysis Period (min) 15							
m Volume for 95th percent	ile queue i	s metereo	l by upstr	eam sign	ial.		



	-	$\mathbf{r}$	1	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				•		1
Volume (vph)	0	0	0	4	0	159
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	0	0	1900	0	1644
Flt Permitted						
Satd. Flow (perm)	0	0	0	1900	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	135			111	668	
Travel Time (s)	9.2			7.6	45.5	
Confl. Peds. (#/hr)						11
Peak Hour Factor	0.92	0.92	0.92	0.33	0.92	0.68
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	0	12	0	234
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	12	0	234
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.4%			IC	U Level o	of Service

Analysis Period (min) 15

	۲	*	<b>\</b>	×	×	4	
Lane Group	WBI	WBR	SEL	SET	NWT	NWR	ø8
Lane Configurations	V DL		JLL		1.000		
Volume (vnh)	Ο	0	1/0	<b>* 1</b> 102	126	100	
Ideal Flow (vph)	1000	1000	147	175	1000	1000	
Lano I Itil Eactor	1 00	1 00	0.05	0.05	1 00	1 00	
Dod Riko Factor	1.00	1.00	0.95	0.90	1.00	1.00	
r eu dike i dului Ert					0.024		
Elt Drotoctod				0.072	0.924		
Satd Elow (prot)	0	0	0	2500	1704	0	
Salu. Flow (plot)	0	0	0	3009	1720	0	
Satd Elow (porm)	0	0	0	2440	1704	0	
Dight Turn on Dod	0	Voc	0	5440	1/20	Voc	
Satd Flow (DTOD)		162			240	162	
Link Spood (mph)	10			10	249		
Link Speed (IIIpII)	10			10	1U 20E		
	107				370		
Traver Time (S)	10.7	41		14.5	26.9		
Conii. Peas. (#/nr)	95	41	0.45	0.00	0.71	0.40	
Peak Hour Factor	0.42	0.35	0.45	0.82	0.71	0.49	
Heavy venicles (%)	0%	0%	0%	0%	4%	0%	
Aaj. Flow (vpn)	U	U	331	235	192	249	
Snared Lane Trattic (%)	2	2	2	<b>F</b> / /		2	
Lane Group Flow (vph)	0	0	0	566	441	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15			9	
Turn Type			Perm	NA	NA		
Protected Phases				6	2		8
Permitted Phases			6				
Minimum Split (s)			3.0	3.0	3.0		3.5
Total Split (s)			3.0	3.0	3.0		3.5
Total Split (%)			46.2%	46.2%	46.2%		54%
Maximum Green (s)			1.0	1.0	1.0		1.0
Yellow Time (s)			2.0	2.0	2.0		2.0
All-Red Time (s)			0.0	0.0	0.0		0.5
Lost Time Adjust (s)				0.0	0.0		
Total Lost Time (s)				2.0	2.0		
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)				1.0	1.0		
Actuated g/C Ratio				0.15	0.15		
v/c Ratio				1.13dl	0.93		
Control Delay				70.7	35.2		
Queue Delay				0.0	0.0		
Total Delay				70.7	35.2		
105				, U.,	D		

Shaw School 4/9/2014 Baseline

	5	*	$\searrow$	X	×	4	
Lane Group	WBL	WBR	SEL	SET	NWT	NWR	ø8
Approach Delay				70.7	35.2		
Approach LOS				E	D		
Stops (vph)				172	59		
Fuel Used(gal)				6	3		
CO Emissions (g/hr)				428	214		
NOx Emissions (g/hr)				83	42		
VOC Emissions (g/hr)				99	50		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	77			132	315		
Turn Bay Length (ft)							
Base Capacity (vph)				530	476		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				1.07	0.93		
Intersection Summary							
Area Type: Of	ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:1	VWT and	6:SETL,	Start of C	Green		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 1.07							
Intersection Signal Delay: 55.2	2			In	tersectior	n LOS: E	
Intersection Capacity Utilization	on 45.1%			IC	U Level o	of Service	А
Analysis Period (min) 15							
dl Defacto Left Lane. Reco	de with 1	though la	ne as a le	eft lane.			

Splits and Phases: 14:

ø2 (R)		
3 s		
96 (R)	● <sub>ø8</sub>	
3 s	3.5 s	

# $\mathbf{A} = \mathbf{A} \mathbf{A} \mathbf{A} \mathbf{A}$

			•			•
Lane Group	EBL	EBR	SBL	SBR	NWL	NWR
Lane Configurations		1	٦	1	ሻ	
Volume (vph)	0	299	47	147	136	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865		0.850		
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	1615	1736	0
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	1615	1736	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	223		93		66	
Travel Time (s)	15.2		6.3		4.5	
Confl. Peds. (#/hr)			58			
Peak Hour Factor	0.92	0.61	0.56	0.42	0.71	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	0	490	84	350	192	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	490	84	350	192	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0	J. J	12	Ŭ	12	Ū
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Stop		Free	
Intersection Summary			•			
	Othor					
Area Type: Control Type: Unclangelized	Uner					
Control Type: Unsignalized						

Intersection Capacity Utilization 21.8% Analysis Period (min) 15

ICU Level of Service A



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	4	$\mathbf{X}$	×	₹	6	*
Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el 🗍		Y	
Volume (vph)	14	133	104	8	9	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.986		0.941	
Flt Protected		0.995			0.973	
Satd. Flow (prot)	0	1890	1809	0	1740	0
Flt Permitted		0.995			0.973	
Satd. Flow (perm)	0	1890	1809	0	1740	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		395	105		180	
Travel Time (s)		26.9	7.2		12.3	
Confl. Peds. (#/hr)	82				118	118
Peak Hour Factor	0.50	0.51	0.65	0.42	0.25	0.46
Heavy Vehicles (%)	0%	0%	4%	0%	0%	0%
Adj. Flow (vph)	28	261	160	19	36	28
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	289	179	0	64	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: 0	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 34.3% Analysis Period (min) 15

ICU Level of Service A

## ۰. ٭ 5 ~ ← EBL EBT WBT SBL Lane Group WBR SBR ø2 **↑**↑ Lane Configurations Volume (vph) 0 0 0 0 116 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 1.00 1.00 1.00 Frt Flt Protected Satd. Flow (prot) 3610 0 0 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 0 3610 0 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) Link Speed (mph) 10 10 10 Link Distance (ft) 157 118 112 Travel Time (s) 10.7 8.0 7.6 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Heavy Vehicles (%) 0% 0% 0% 0% 0% 0% Adj. Flow (vph) 126 0 0 0 0 0 Shared Lane Traffic (%) 0 Lane Group Flow (vph) 0 126 0 0 0 Enter Blocked Intersection No No No No No No Lane Alignment Left Left Left Right Left Right Median Width(ft) 0 0 0 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 NA Turn Type Protected Phases 2 4 Permitted Phases Minimum Split (s) 3.0 3.5 Total Split (s) 3.0 3.5 Total Split (%) 46.2% 54% Maximum Green (s) 1.0 1.0 Yellow Time (s) 2.0 2.0 All-Red Time (s) 0.0 0.5 Lost Time Adjust (s) 0.0 Total Lost Time (s) 2.0 Lead/Lag Lead-Lag Optimize? Act Effct Green (s) 1.0 Actuated g/C Ratio 0.15 v/c Ratio 0.23 Control Delay 3.6 Queue Delay 0.0 Total Delay 3.6 LOS А Approach Delay 3.6 Approach LOS А

Shaw School 4/9/2014 Baseline

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø2		
Stops (vph)		12							
Fuel Used(gal)		0							
CO Emissions (g/hr)		21							
NOx Emissions (g/hr)		4							
VOC Emissions (g/hr)		5							
Dilemma Vehicles (#)		0							
Queue Length 50th (ft)		0							
Queue Length 95th (ft)		m0							
Internal Link Dist (ft)		77	38		32				
Turn Bay Length (ft)									
Base Capacity (vph)		555							
Starvation Cap Reductn		0							
Spillback Cap Reductn		0							
Storage Cap Reductn		0							
Reduced v/c Ratio		0.23							
Intersection Summary									
Area Type: O	)ther								
Cycle Length: 6.5									
Actuated Cycle Length: 6.5									
Offset: 0 (0%), Referenced to	phase 2:H	old and	6:, Start o	of Green					
Natural Cycle: 40									
Control Type: Pretimed									
Maximum v/c Ratio: 0.23									
Intersection Signal Delay: 3.6	)			Int	ersection	LOS: A			
Intersection Capacity Utilization	on 6.7%			IC	U Level of	f Service	А		
Analysis Period (min) 15									
m Volume for 95th percenti	le queue is	metered	l by upstr	eam signa	al.				
Splits and Phases: 2:									

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Lane Group	WBL	WBR	NBL	NBR	SEL	SER			
Lane Configurations	Y		Y		Y				
Volume (vph)	38	65	47	23	94	48			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Ped Bike Factor									
Frt	0.922		0.965		0.954				
Flt Protected	0.979		0.964		0.968				
Satd. Flow (prot)	1715	0	1693	0	1755	0			
Flt Permitted	0.979		0.964		0.968				
Satd. Flow (perm)	1715	0	1693	0	1755	0			
Link Speed (mph)	10		10		10				
Link Distance (ft)	408		65		105				
Travel Time (s)	27.8		4.4		7.2				
Confl. Peds. (#/hr)			47			82			
Confl. Bikes (#/hr)				47					
Peak Hour Factor	0.53	0.65	0.38	0.52	0.52	0.51			
Heavy Vehicles (%)	0%	0%	6%	0%	0%	0%			
Adj. Flow (vph)	72	100	124	44	181	94			
Shared Lane Traffic (%)									
Lane Group Flow (vph)	172	0	168	0	275	0			
Enter Blocked Intersection	No	No	No	No	No	No			
Lane Alignment	Left	Right	Left	Right	Left	Right			
Median Width(ft)	12		12		12				
Link Offset(ft)	0		0		0				
Crosswalk Width(ft)	16		16		16				
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Turning Speed (mph)	15	9	15	9	15	9			
Sign Control	Free		Stop		Free				
Intersection Summary									
Area Type:	Other								
Control Type: Unsignalized									
Intersection Capacity Utilization	tion 33.2%			IC	CU Level o	of Service A	А		
Analysis Period (min) 15									

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8	
Lane Configurations			*			*		
Volume (vnh)	0	0	70	0	0	86		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
ane Util Factor	1 00	1 00	1 00	1 00	1 00	1.00		
-rt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	1827	0	0	1900		
Elt Permitted	Ū	U	1027	U	U	1700		
Satd Flow (perm)	0	0	1827	0	0	1900		
Right Turn on Red	Ū	Yes	1027	Yes	U	1700		
Satd Flow (RTOR)		105		105				
ink Sneed (mnh)	10		10			10		
ink Distance (ft)	228		261			65		
ravel Time (s)	15.5		17.8			4 4		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92		
leavy Vehicles (%)	0.72	0.72	1%	0.72	0.72	0.72		
Adi Flow (vnh)	0.0	070	76	070	070	070		
Shared Lane Traffic (%)	U	U	70	U	U	75		
ane Group Flow (vph)	0	Ο	76	0	Ο	02		
Enter Blocked Intersection	No	No	No	No	No	73 No		
ane Alianment	Loft	Right	Loft	Right	I oft	l off		
And Alignment Addian Width(ft)		Right		Right	LEII			
ink Offsot(ft)	0		0			0		
Circoswalk Width(ft)	16		16			16		
Two way Loft Turn Lano	10		10			10		
WU Way Left Turri Lane	1 00	1 00	1 00	1.00	1 00	1.00		
Turning Spood (mph)	1.00	1.00	1.00	1.00	1.00	1.00		
urning Speed (mpn)	15	9	NIA	9	15	NIA		
un Type Protoctod Dhasos						NA 4	0	
Pormitted Phases			Z			0	0	
Ainimum Split (c)			2.0			2.0	2 5	
Total Split (s)			3.U 2.O			3.U 2.O	3.0 2.5	
otal Split (%)			3.0			3.U 16 20/	5.5	
Javimum Croon (c)			40.270 1.∩			40.2 <i>%</i>	10	
Vallow Time (s)			1.0			1.0	1.0	
UII Dod Timo (s)			2.0			2.0	2.0	
ast Time Adjust (s)			0.0			0.0	0.5	
LUSE TIME AUJUSE (S)			0.0			0.0		
			2.0			2.0		
ead Lag Optimize?								
eau-Lay Optimize?			1.0			1.0		
ALL EIILL GIEEII (S)			1.U			1.U		
Nciualeu y/C Kallu			0.15			0.15		
/L KallO			0.27			0.32		
Jonitol Delay			4.9			4.5		
Lueue Delay			0.0			0.0		
olai Delay			4.9			4.5		
_US			A			A		
Approach Delay			4.9			4.5		
Approach LOS			A			A		

Shaw School 4/9/2014 Baseline

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Stops (vph)			9			8	
Fuel Used(gal)			0			0	
CO Emissions (g/hr)			20			10	
NOx Emissions (g/hr)			4			2	
VOC Emissions (g/hr)			5			2	
Dilemma Vehicles (#)			0			0	
Queue Length 50th (ft)			0			0	
Queue Length 95th (ft)			0			m0	
Internal Link Dist (ft)	148		181			1	
Turn Bay Length (ft)							
Base Capacity (vph)			281			292	
Starvation Cap Reductn			0			0	
Spillback Cap Reductn			0			0	
Storage Cap Reductn			0			0	
Reduced v/c Ratio			0.27			0.32	
Intersection Summary							
Area Type: O	ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:	NBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 0.32							
Intersection Signal Delay: 4.7				Int	tersection	LOS: A	
Intersection Capacity Utilization	on 7.9%			IC	U Level o	f Service A	4
Analysis Period (min) 15							
m Volume for 95th percentil	le queue i	s meterec	l by upstr	eam sign	al.		
Culita and Dhasas. 7.							


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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				•		1
Volume (vph)	0	0	0	25	0	117
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	0	0	1900	0	1644
Flt Permitted						
Satd. Flow (perm)	0	0	0	1900	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	135			111	668	
Travel Time (s)	9.2			7.6	45.5	
Confl. Peds. (#/hr)						11
Peak Hour Factor	0.92	0.92	0.92	0.39	0.92	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	0	64	0	225
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	64	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.4%			IC	U Level of	of Service

Analysis Period (min) 15

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	- \\//DI		CEI	CET.			α <sup>0</sup>	
Lane Configurations	VVDL	WDR	JEL				00	
	0	0	77	<b>M T</b>	79	20		
Idoal Elow (vphpl)	1000	1000	1000	1000	1000	1000		
Lano Litil Eactor	1 00	1,00	0.05	0.05	1,00	1,00		
Dod Piko Eactor	1.00	1.00	0.95	0.95	0.00	1.00		
				0.77	0.77			
				0.001	0.949			
Satd Elow (prot)	0	0	0	2552	1716	0		
Elt Dormittod	0	0	0	0.055	1740	0		
Satd Flow (porm)	0	0	٥	2255	17/6	٥		
Pight Turn on Red	0	Vas	U	1111	1740	Vas		
Satd Flow (PTOP)		163			56	163		
Link Sneed (mph)	10			10	10			
Link Distance (ff)	157			212	205			
Travel Time (s)	10.7			1/ 5	26.0			
Confl Peds (#/hr)	177	52	52	14.5	20.7	1		
Peak Hour Factor	0.92	0 92	0.7/	0.68	0.85	0 70		
Heavy Vehicles (%)	0.72	0.72	0.74	0.00	4%	0.70		
Adi Flow (vnh)	070	070	10/0	216	470	56		
Shared Lane Traffic (%)	U	U	104	210	72	50		
Lane Group Flow (vph)	0	0	0	320	148	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rtight	Lon	0	0	rtigrit		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane	10				10			
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type		,	Perm	NA	NA	,		
Protected Phases				6	2		8	
Permitted Phases			6	3	_			
Minimum Split (s)			3.0	3.0	3.0		3.5	
Total Split (s)			3.0	3.0	3.0		3.5	
Total Split (%)			46.2%	46.2%	46.2%		54%	
Maximum Green (s)			1.0	1.0	1.0		1.0	
Yellow Time (s)			2.0	2.0	2.0		2.0	
All-Red Time (s)			0.0	0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag				2.0	2.0			
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				0.62	0.47			
Control Delay				8.9	6.7			
Queue Delay				0.0	0.0			
Total Delay				8.9	6.7			
LOS				A	A			

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Lane Group	WBL	WBR	SEL	SET	NWT	NWR	ø8	
Approach Delay				8.9	6.7			
Approach LOS				А	А			
Stops (vph)				33	13			
Fuel Used(gal)				1	1			
CO Emissions (g/hr)				80	49			
NOx Emissions (g/hr)				16	10			
VOC Emissions (g/hr)				19	11			
Dilemma Vehicles (#)				0	0			
Queue Length 50th (ft)				0	0			
Queue Length 95th (ft)				0	0			
Internal Link Dist (ft)	77			132	315			
Turn Bay Length (ft)								
Base Capacity (vph)				516	316			
Starvation Cap Reductn				0	0			
Spillback Cap Reductn				0	0			
Storage Cap Reductn				0	0			
Reduced v/c Ratio				0.62	0.47			
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced	to phase 2:	NWT and	6:SETL,	Start of C	Green			
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 0.62								
Intersection Signal Delay: 8	.2			In	itersectior	n LOS: A		
Intersection Capacity Utiliza	ation 29.2%			IC	CU Level	of Service	A	
Analysis Period (min) 15								
Splits and Phases: 14:								

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3s		
3 s ≪ ø6 (R)	ø8 3.5 s	



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Lane Configurations				**				
Volume (vph)	0	0	0	379	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00		
Frt								
Flt Protected								
Satd. Flow (prot)	0	0	0	3574	0	0		
Flt Permitted								
Satd. Flow (perm)	0	0	0	3574	0	0		
Right Turn on Red	Yes	Yes	Yes			Yes		
Satd. Flow (RTOR)								
Link Speed (mph)	10			10	10			
Link Distance (ft)	76			150	44			
Travel Time (s)	5.2			10.2	3.0			
Peak Hour Factor	0.92	0.92	0.92	0.45	0.92	0.92		
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%		
Adi, Flow (vph)	0	0	0	842	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	842	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0			0	0			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type				NA				
Protected Phases				2			4	
Permitted Phases								
Minimum Split (s)				3.0			3.5	
Total Split (s)				3.0			3.5	
Total Split (%)				46.2%			54%	
Maximum Green (s)				1.0			1.0	
Yellow Time (s)				2.0			2.0	
All-Red Time (s)				0.0			0.5	
Lost Time Adjust (s)				0.0				
Total Lost Time (s)				2.0				
Lead/Lag								
Lead-Lag Optimize?								
Act Effct Green (s)				1.0				
Actuated g/C Ratio				0.15				
v/c Ratio				1.53				
Control Delay				264.5				
Queue Delay				0.0				
Total Delay				264.5				
LOS				F				
Approach Delay				264.5				
Approach LOS				F				

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Stops (vph)				685				
Fuel Used(gal)				23				
CO Emissions (g/hr)				1584				
NOx Emissions (g/hr)				308				
VOC Emissions (g/hr)				367				
Dilemma Vehicles (#)				0				
Queue Length 50th (ft)				~15				
Queue Length 95th (ft)				0				
Internal Link Dist (ft)	1			70	1			
Turn Bay Length (ft)								
Base Capacity (vph)				549				
Starvation Cap Reductn				0				
Spillback Cap Reductn				0				
Storage Cap Reductn				0				
Reduced v/c Ratio				1.53				
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced t	to phase 2:I	<b>VBT</b> and	6:, Start (	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 1.53								
Intersection Signal Delay: 20	64.5			Int	ersection	LOS: F		
Intersection Capacity Utiliza	tion 20.5%			IC	U Level o	f Service	A	
Analysis Period (min) 15								
<ul> <li>Volume exceeds capacity</li> </ul>	ty, queue is	theoretic	ally infini	te.				
Queue shown is maximu	m after two	cycles.						
Splits and Phases: 2:								
Ø2 (R)					ø4			

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	60	205	312	41	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.983		0.973	
Flt Protected		0.988			0.962	
Satd. Flow (prot)	0	1877	1820	0	1778	0
Flt Permitted		0.988			0.962	
Satd. Flow (perm)	0	1877	1820	0	1778	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		379	107		88	
Travel Time (s)		25.8	7.3		6.0	
Confl. Peds. (#/hr)	80			2	51	
Peak Hour Factor	0.45	0.51	0.67	0.62	0.25	0.50
Heavy Vehicles (%)	0%	0%	3%	0%	0%	0%
Adj. Flow (vph)	133	402	466	66	32	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	535	532	0	40	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: (	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 46.4% Analysis Period (min) 15

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	4Î			
Volume (vph)	213	215	150	166	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.917			
Flt Protected		0.969				
Satd. Flow (prot)	0	1841	1700	0	0	0
Flt Permitted		0.969				
Satd. Flow (perm)	0	1841	1700	0	0	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		263	379		267	
Travel Time (s)		17.9	25.8		18.2	
Confl. Peds. (#/hr)	41			95	95	
Peak Hour Factor	0.45	0.82	0.71	0.49	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	0%	0%
Adj. Flow (vph)	473	262	211	339	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	735	550	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Free	
Intersection Summary						
Area Type: C	Other					

Control Type: Unsignalized Intersection Capacity Utilization 50.9% Analysis Period (min) 15

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Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR
Lane Configurations					र्स	ef 👘	
Volume (vph)	0	0	100	71	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	0	0	0	1805	1900	0
Flt Permitted					0.950		
Satd. Flow (perm)	0	0	0	0	1805	1900	0
Link Speed (mph)	10				10	10	
Link Distance (ft)	558				79	105	
Travel Time (s)	38.0				5.4	7.2	
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	250	178	0	0	0
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	0	428	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Right	R NA	Left	Left	Left	Right
Median Width(ft)	0				0	0	
Link Offset(ft)	0				0	0	
Crosswalk Width(ft)	16				16	16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15			9
Sign Control	Yield				Yield	Yield	
Intersection Summary							
Area Type:	Other						
Control Type: Roundabout							
Intersection Capacity Utiliza	ation 12.8%			IC	CU Level o	of Service	A
Analysis Period (min) 15							

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	•			<del>ا</del>
Volume (vph)	0	70	101	0	88	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865				
Flt Protected						0.958
Satd. Flow (prot)	0	1644	1900	0	0	1820
Flt Permitted						0.958
Satd. Flow (perm)	0	1644	1900	0	0	1820
Link Speed (mph)	10		10			10
Link Distance (ft)	164		264			132
Travel Time (s)	11.2		18.0			9.0
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	175	253	0	220	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	175	252	0	0	250
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Yield		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 16.3% Analysis Period (min) 15

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Lane Group	EBL	EBR	NWL	NWR	SWL	SWR
Lane Configurations		1	۲.		۲.	1
Volume (vph)	0	409	150	0	69	257
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	0	1805	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	0	1805	1583
Link Speed (mph)	10		10		10	
Link Distance (ft)	88		263		95	
Travel Time (s)	6.0		17.9		6.5	
Confl. Peds. (#/hr)		1			58	
Peak Hour Factor	0.92	0.61	0.71	0.92	0.56	0.42
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%
Adj. Flow (vph)	0	670	211	0	123	612
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	670	211	0	123	612
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	tion 28.8%			IC	CU Level	of Service

Analysis Period (min) 15

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Lane Group	WBL	WBR	SEL	SER	NEL	NER
Lane Configurations	Y		¥.		Ý	
Volume (vph)	41	200	147	66	153	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.889		0.950		0.985	
Flt Protected	0.991		0.970		0.957	
Satd. Flow (prot)	1674	0	1751	0	1729	0
Flt Permitted	0.991		0.970		0.957	
Satd. Flow (perm)	1674	0	1751	0	1729	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	379		107		213	
Travel Time (s)	25.8		7.3		14.5	
Confl. Peds. (#/hr)	2	2	80	80	28	28
Peak Hour Factor	0.63	0.67	0.68	0.51	0.39	0.65
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	65	299	216	129	392	48
Shared Lane Traffic (%)						
Lane Group Flow (vph)	364	0	345	0	440	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type: (	Other					

Control Type: Unsignalized Intersection Capacity Utilization 51.0% Analysis Period (min) 15

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Lane Group	FBI	FBR	NBI	NBT	SBT	SBR	ø4	
Lane Configurations		LDIX	nd L			ODIX	~ 1	
Volume (vnh)	0	0	0	171	100	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane I Itil Factor	1.00	1 00	1 00	1 00	1.00	1.00		
Frt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	0	1900	1900	0		
Elt Permitted	0	U	0	1700	1700	U		
Satd Flow (perm)	0	0	0	1900	1900	0		
Right Turn on Red	U	Yes	0	1700	1700	Yes		
Satd Flow (RTOR)		105				105		
Link Speed (mph)	30			30	30			
Link Distance (ft)	145			132	79			
Travel Time (s)	33			3.0	18			
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%		
Adi Flow (vph)	0	0	0	428	250	0		
Shared Lane Traffic (%)	Ū	Ū	J	120	200	U		
Lane Group Flow (vph)	0	0	0	428	250	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rtigrit	Lon	0	0	rtigitt		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type		·		NA	NA			
Protected Phases				2	6		4	
Permitted Phases				_				
Minimum Split (s)				3.0	3.0		3.5	
Total Split (s)				3.0	3.0		3.5	
Total Split (%)				46.2%	46.2%		54%	
Maximum Green (s)				1.0	1.0		1.0	
Yellow Time (s)				2.0	2.0		2.0	
All-Red Time (s)				0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag								
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				1.47	0.86			
Control Delay				243.8	37.4			
Queue Delay				0.0	0.0			
Total Delay				243.8	37.4			
LOS				F	D			
Approach Delay				243.8	37.4			
Approach LOS				F	D			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4
Stops (vph)				283	28		
Fuel Used(gal)				10	1		
CO Emissions (g/hr)				715	68		
NOx Emissions (g/hr)				139	13		
VOC Emissions (g/hr)				166	16		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	65			52	1		
Turn Bay Length (ft)							
Base Capacity (vph)				292	292		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				1.47	0.86		
Intersection Summary							
Area Type: Of	ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:1	VBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 1.47							
Intersection Signal Delay: 167	7.7			Int	ersection	LOS: F	
Intersection Capacity Utilization	on 12.3%			IC	U Level o	of Service	A
Analysis Period (min) 15							

Splits and Phases: 19:

∫ ø2 (R)	<b>e</b> <sub>ø4</sub>	
3 s	3.5 s	
∮ ø6 (R)		
3 s		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4Î			र्स		1	
Volume (vph)	44	44	197	70	0	178	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.932					0.865	
Flt Protected				0.964			
Satd. Flow (prot)	1771	0	0	1832	0	1644	
Flt Permitted				0.964			
Satd. Flow (perm)	1771	0	0	1832	0	1644	
Link Speed (mph)	10			10	10		
Link Distance (ft)	164			348	551		
Travel Time (s)	11.2			23.7	37.6		
Confl. Peds. (#/hr)		11					
Peak Hour Factor	0.33	0.33	0.33	0.33	0.92	0.33	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	133	133	597	212	0	539	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	266	0	0	809	0	539	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Canacity Utiliza	tion 25.6%			IC	: LL evel (	of Service	Δ

Analysis Period (min) 15

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations			<b>†</b> †			1	
Volume (vph)	0	0	379	0	0	50	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00	
Frt						0.865	
Flt Protected							
Satd. Flow (prot)	0	0	3539	0	0	1611	
Flt Permitted							
Satd. Flow (perm)	0	0	3539	0	0	1611	
Link Speed (mph)		10	10		30		
Link Distance (ft)		64	172		141		
Travel Time (s)		4.4	11.7		3.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	412	0	0	54	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	412	0	0	54	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Free		
Intersection Summary							
Area Type: 0	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 20.5%			IC	U Level	of Service	ЭA
Analysis Period (min) 15							

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Lane Group	EBL	EBR	SBL	SBR	NWL	NWR
Lane Configurations		1	۲	1	۲	
Volume (vph)	0	189	38	88	78	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865		0.850		
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	1615	1736	0
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	1615	1736	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	223		93		66	
Travel Time (s)	15.2		6.3		4.5	
Confl. Peds. (#/hr)			104			
Peak Hour Factor	0.92	0.82	0.68	0.46	0.88	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	0	230	56	191	89	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	230	56	191	89	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Stop		Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 15.0% Analysis Period (min) 15



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Lane Configurations				44				
Volume (vph)	0	0	0	214	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util Factor	1 00	1 00	1 00	0.95	1 00	1 00		
Frt				0170				
Flt Protected								
Satd, Flow (prot)	0	0	0	3574	0	0		
Flt Permitted	Ũ	Ū	Ŭ	0071	Ū	Ŭ		
Satd, Flow (perm)	0	0	0	3574	0	0		
Right Turn on Red	Yes	Yes	Yes	0071	Ū	Yes		
Satd Flow (RTOR)								
Link Speed (mph)	10			10	10			
Link Distance (ff)	76			150	44			
Travel Time (s)	5.2			10.2	3.0			
Peak Hour Factor	0.92	0.92	0.92	0.45	0.92	0.92		
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%		
Adi, Flow (vph)	0	0	0	476	0	0		
Shared Lane Traffic (%)	Ŭ	Ű	Ű	110	Ű	Ū		
Lane Group Flow (vph)	0	0	0	476	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rugni	Eon	0	0	rtigitt		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane	10			10	10			
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15	1.00	1.00	9		
Turn Type	10	,		NA		,		
Protected Phases				2			4	
Permitted Phases				-			•	
Minimum Split (s)				3.0			35	
Total Split (s)				3.0			3.5	
Total Split (%)				46.2%			54%	
Maximum Green (s)				10.270			10	
Yellow Time (s)				2.0			2.0	
All-Red Time (s)				0.0			0.5	
Lost Time Adjust (s)				0.0			0.0	
Total Lost Time (s)				2.0				
Lead/Lag				2.0				
Lead-Lag Ontimize?								
Act Effct Green (s)				10				
Actuated g/C Ratio				0.15				
v/c Ratio				0.10				
Control Delay				24.9				
Oueue Delay				0.0				
Total Delay				24.9				
105				<u> </u>				
Approach Delay				24.9				
Approach LOS				C				

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Stops (vph)				47				
Fuel Used(gal)				2				
CO Emissions (g/hr)				150				
NOx Emissions (g/hr)				29				
VOC Emissions (g/hr)				35				
Dilemma Vehicles (#)				0				
Queue Length 50th (ft)				0				
Queue Length 95th (ft)				0				
Internal Link Dist (ft)	1			70	1			
Turn Bay Length (ft)								
Base Capacity (vph)				549				
Starvation Cap Reductn				0				
Spillback Cap Reductn				0				
Storage Cap Reductn				0				
Reduced v/c Ratio				0.87				
Intersection Summary								
Area Type: C	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced to	o phase 2:1	NBT and	6:, Start o	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 0.87								
Intersection Signal Delay: 24	.9			Int	tersection	LOS: C		
Intersection Capacity Utilizati	ion 9.2%			IC	U Level o	f Service	A	
Analysis Period (min) 15								

Splits and Phases: 2:

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	15	146	145	22	9	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.974		0.941	
Flt Protected		0.995			0.973	
Satd. Flow (prot)	0	1890	1807	0	1740	0
Flt Permitted		0.995			0.973	
Satd. Flow (perm)	0	1890	1807	0	1740	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		379	107		88	
Travel Time (s)		25.8	7.3		6.0	
Confl. Peds. (#/hr)	82				118	
Peak Hour Factor	0.50	0.51	0.65	0.42	0.25	0.46
Heavy Vehicles (%)	0%	0%	3%	0%	0%	0%
Adj. Flow (vph)	30	286	223	52	36	28
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	316	275	0	64	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: 0	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 30.2% Analysis Period (min) 15

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्च	el el			
Volume (vph)	147	161	91	67	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.936			
Flt Protected		0.978				
Satd. Flow (prot)	0	1858	1745	0	0	0
Flt Permitted		0.978				
Satd. Flow (perm)	0	1858	1745	0	0	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		263	379		267	
Travel Time (s)		17.9	25.8		18.2	
Confl. Peds. (#/hr)	52			4	177	
Peak Hour Factor	0.74	0.68	0.85	0.70	0.42	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	0%	0%
Adj. Flow (vph)	199	237	107	96	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	436	203	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Free	
Intersection Summary						
Area Type: (	Other					

Control Type: Unsignalized Intersection Capacity Utilization 32.9% Analysis Period (min) 15

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Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR	
Lane Configurations					र्स	el el		
Volume (vph)	0	0	74	28	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt								
Flt Protected					0.950			
Satd. Flow (prot)	0	0	0	0	1805	1900	0	
Flt Permitted					0.950			
Satd. Flow (perm)	0	0	0	0	1805	1900	0	
Link Speed (mph)	10				10	10		
Link Distance (ft)	92				79	105		
Travel Time (s)	6.3				5.4	7.2		
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40	0.40	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	0	0	185	70	0	0	0	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	0	255	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	No	
Lane Alignment	Left	Right	R NA	Left	Left	Left	Right	
Median Width(ft)	0				0	0		
Link Offset(ft)	0				0	0		
Crosswalk Width(ft)	16				16	16		
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	9	15			9	
Sign Control	Yield				Yield	Yield		
Intersection Summary								
Area Type:	Other							
Control Type: Roundabout								
Intersection Capacity Utiliza	tion 9.0%			IC	CU Level o	of Service	A	
Analysis Period (min) 15								

	4	•	1	1	1	Ŧ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	<b>†</b>			નુ
Volume (vph)	0	65	37	0	52	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865				
Flt Protected						0.966
Satd. Flow (prot)	0	1644	1900	0	0	1835
Flt Permitted						0.966
Satd. Flow (perm)	0	1644	1900	0	0	1835
Link Speed (mph)	10		10			10
Link Distance (ft)	164		264			132
Travel Time (s)	11.2		18.0			9.0
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	163	93	0	130	55
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	162	92	0	0	185
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Yield		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 14.0% Analysis Period (min) 15

	_#	-*	*	₹.	L.	~	
Lane Group	EBL	EBR	NWL	NWR	SWL	SWR	
Lane Configurations		1	5		5	1	
Volume (vph)	0	256	91	0	52	154	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt		0.865				0.850	
Flt Protected			0.950		0.950		
Satd. Flow (prot)	0	1644	1805	0	1805	1583	
Flt Permitted			0.950		0.950		
Satd. Flow (perm)	0	1644	1805	0	1805	1583	
Link Speed (mph)	10		10		10		
Link Distance (ft)	88		263		95		
Travel Time (s)	6.0		17.9		6.5		
Confl. Peds. (#/hr)					104		
Peak Hour Factor	0.92	0.82	0.88	0.92	0.68	0.46	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	
Adj. Flow (vph)	0	312	103	0	76	335	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	312	103	0	76	335	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	0		12		12		
Link Offset(ft)	0		0		0		
Crosswalk Width(ft)	16		16		16		
I wo way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15	9	15	9	
Sign Control	Free		Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utilization	tion 19.2%			IC	CU Level	of Service	λ

Analysis Period (min) 15

	*	*	$\searrow$	2	3	/
Lane Group	WBL	WBR	SEL	SER	NEL	NER
Lane Configurations	Y		Y		Y	
Volume (vph)	51	92	102	53	75	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.906		0.953		0.975	
Flt Protected	0.985		0.968		0.961	
Satd. Flow (prot)	1696	0	1753	0	1724	0
Flt Permitted	0.985		0.968		0.961	
Satd. Flow (perm)	1696	0	1753	0	1724	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	379		107		213	
Travel Time (s)	25.8		7.3		14.5	
Confl. Peds. (#/hr)		2	82		47	
Peak Hour Factor	0.53	0.42	0.52	0.51	0.38	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	96	219	196	104	197	44
Shared Lane Traffic (%)						
Lane Group Flow (vph)	315	0	300	0	241	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12	Ū	12	Ū	12	Ū
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type: (	Other					
Control Type, Unclandized						

Control Type: Unsignalized Intersection Capacity Utilization 33.4% Analysis Period (min) 15

	۶	$\mathbf{r}$	•	1	Ļ	-		
Lane Group	FBI	FBR	NBI	NBT	SBT	SBR	ø4	
Lane Configurations		LDIX	NDL			ODIX	01	
Volume (vnh)	0	0	0	102	7/	0		
Ideal Flow (vph)	1900	1900	1900	1900	1900	1900		
Lane Util Eactor	1 00	1 00	1 00	1 00	1 00	1 00		
Frt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	0	1900	1900	0		
Elt Permitted	U	U	0	1700	1700	U		
Satd Flow (perm)	0	0	0	1900	1900	0		
Right Turn on Red	U	Yes	0	1700	1700	Yes		
Satd Flow (RTOR)		105				105		
Link Speed (mph)	30			30	30			
Link Distance (ft)	145			132	79			
Travel Time (s)	3.3			3.0	1.8			
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%		
Adi, Flow (vph)	0	0	0	255	185	0		
Shared Lane Traffic (%)	0	Ū	U	200	100	Ū		
Lane Group Flow (vnh)	0	0	0	255	185	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0			0	0			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type				NA	NA			
Protected Phases				2	6		4	
Permitted Phases				_	-			
Minimum Split (s)				3.0	3.0		3.5	
Total Split (s)				3.0	3.0		3.5	
Total Split (%)				46.2%	46.2%		54%	
Maximum Green (s)				1.0	1.0		1.0	
Yellow Time (s)				2.0	2.0		2.0	
All-Red Time (s)				0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag								
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				0.87	0.63			
Control Delay				40.1	16.3			
Queue Delay				0.0	0.0			
Total Delay				40.1	16.3			
LOS				D	В			
Approach Delay				40.1	16.3			
Approach LOS				D	В			

	۶	$\mathbf{F}$	•	t	Ļ	∢	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4
Stops (vph)				30	13		
Fuel Used(gal)				1	0		
CO Emissions (g/hr)				77	25		
NOx Emissions (g/hr)				15	5		
VOC Emissions (g/hr)				18	6		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	65			52	1		
Turn Bay Length (ft)							
Base Capacity (vph)				292	292		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				0.87	0.63		
Intersection Summary							
Area Type: Ot	her						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:I	NBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 0.87							
Intersection Signal Delay: 30.1				Int	ersection	LOS: C	
Intersection Capacity Utilizatio	n 8.7%			IC	U Level o	f Service A	4
Analysis Period (min) 15							

Splits and Phases: 19:

Ø2 (R)	<b>e</b> ø4	
3 s	3.5 s	
∮ ∮ ø6 (R)		
3 s		

	-	$\mathbf{r}$	1	+	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	eî.			र्स		1
Volume (vph)	26	26	117	65	0	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932					0.865
Flt Protected				0.967		
Satd. Flow (prot)	1771	0	0	1837	0	1644
Flt Permitted				0.967		
Satd. Flow (perm)	1771	0	0	1837	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	164			348	551	
Travel Time (s)	11.2			23.7	37.6	
Confl. Peds. (#/hr)		25				
Peak Hour Factor	0.33	0.33	0.33	0.39	0.92	0.33
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	79	79	355	167	0	379
Shared Lane Traffic (%)						
Lane Group Flow (vph)	158	0	0	522	0	379
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 23.7%			IC	U Level	of Service

Intersection Capacity Utili Analysis Period (min) 15

## **APPENDIX C**

MIDDLE AND ELEMENTARY SCHOOLS OPERATIONAL ANALYSIS CALCULATIONS



bing @2011 Microsoft Corporation @AND @2010 NAVTEQ Shaw School Baseline\13630MB Swampscott School Traffic\Synchro\13630MB\_Swampscott\_AM\_No-Build\_042414.syn

	-	$\mathbf{x}$	×	ť	6	*
Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	9	182	246	19	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.990		0.973	
Flt Protected		0.997			0.962	
Satd. Flow (prot)	0	1894	1814	0	1778	0
Flt Permitted		0.997			0.962	
Satd. Flow (perm)	0	1894	1814	0	1778	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		395	105		180	
Travel Time (s)		26.9	7.2		12.3	
Confl. Peds. (#/hr)	80				51	
Peak Hour Factor	0.45	0.68	0.67	0.63	0.25	0.50
Heavy Vehicles (%)	0%	0%	4%	0%	0%	0%
Adj. Flow (vph)	20	268	367	30	32	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	288	397	0	40	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 26.9% Analysis Period (min) 15

## ۰. ٭ 5 ~ ← EBL EBT WBT SBL Lane Group WBR SBR ø2 **↑**↑ Lane Configurations Volume (vph) 0 271 0 0 0 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 1.00 1.00 1.00 Frt Flt Protected Satd. Flow (prot) 3610 0 0 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 0 3610 0 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) Link Speed (mph) 10 10 10 Link Distance (ft) 157 118 112 Travel Time (s) 10.7 8.0 7.6 Peak Hour Factor 0.92 0.92 0.92 0.92 0.45 0.92 Heavy Vehicles (%) 0% 0% 0% 0% 0% 0% Adj. Flow (vph) 602 0 0 0 0 0 Shared Lane Traffic (%) 0 0 Lane Group Flow (vph) 602 0 0 0 Enter Blocked Intersection No No No No No No Lane Alignment Left Left Left Right Left Right Median Width(ft) 0 0 0 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 NA Turn Type Protected Phases 2 4 Permitted Phases Minimum Split (s) 3.0 3.5 Total Split (s) 3.0 3.5 Total Split (%) 46.2% 54% Maximum Green (s) 1.0 1.0 Yellow Time (s) 2.0 2.0 All-Red Time (s) 0.0 0.5 Lost Time Adjust (s) 0.0 Total Lost Time (s) 2.0 Lead/Lag Lead-Lag Optimize? Act Effct Green (s) 1.0 Actuated g/C Ratio 0.15 v/c Ratio 1.08 Control Delay 60.2 Queue Delay 0.0 Total Delay 60.2 LOS Ε 60.2 Approach Delay

Shaw School 4/9/2014 Baseline

Ε

Approach LOS

	≯	-	-	*	1	<		
ane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø2	
Stops (vph)		115						
Fuel Used(gal)		4						
CO Emissions (g/hr)		271						
NOx Emissions (g/hr)		53						
VOC Emissions (g/hr)		63						
Dilemma Vehicles (#)		0						
Queue Length 50th (ft)		0						
Queue Length 95th (ft)		0						
Internal Link Dist (ft)		77	38		32			
Turn Bay Length (ft)								
Base Capacity (vph)		555						
Starvation Cap Reductn		0						
Spillback Cap Reductn		0						
Storage Cap Reductn		0						
Reduced v/c Ratio		1.08						
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced t	to phase 2:F	Hold and	6:, Start	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 1.08								
Intersection Signal Delay: 60	0.2			In	tersection	LOS: E		
Intersection Capacity Utiliza	tion 10.8%			IC	U Level o	of Service	ł	
Analysis Period (min) 15								
Splits and Phases: 2:								

ø2 (R)		<b>→</b> ø4	
3.5 s		3 s	

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Lane Group	WBL	WBR	NBL	NBR	SEL	SER	
Lane Configurations	¥		Y		Y		
Volume (vph)	19	156	109	31	131	59	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.880		0.980		0.949		
Flt Protected	0.994		0.959		0.970		
Satd. Flow (prot)	1662	0	1699	0	1749	0	
Flt Permitted	0.994		0.959		0.970		
Satd. Flow (perm)	1662	0	1699	0	1749	0	
Link Speed (mph)	10		10		10		
Link Distance (ft)	408		65		105		
Travel Time (s)	27.8		4.4		7.2		
Confl. Peds. (#/hr)	28	2	28			28	
Peak Hour Factor	0.59	0.63	0.39	0.65	0.68	0.51	
Heavy Vehicles (%)	0%	0%	6%	0%	0%	0%	
Adj. Flow (vph)	32	248	279	48	193	116	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	280	0	327	0	309	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	12		12		12		
Link Offset(ft)	0		0		0		
Crosswalk Width(ft)	16		16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	15	9	15	9	
Sign Control	Free		Stop		Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 41.6%			IC	CU Level of	of Service A	A

Analysis Period (min) 15

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	ø8		
Lane Configurations			*			*			
Volume (vph)	0	0	140	0	0	76			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Frt									
Flt Protected									
Satd Flow (prot)	0	0	1827	0	0	1900			
-It Permitted		Ŭ		0	Ű				
Satd Flow (perm)	0	0	1827	0	0	1900			
Right Turn on Red	Ū	Yes	1027	Yes	Ű	1700			
Satd Flow (RTOR)		105		100					
ink Speed (mph)	10		10			10			
ink Distance (ft)	228		261			65			
Fravel Time (s)	15 5		17.8			4 4			
Peak Hour Factor	0.92	0.92	0.39	0.92	0.92	0.51			
Heavy Vehicles (%)	0.72	0%	4%	0%	0%	0%			
Adi Flow (vnh)	070	0,0	250	070	070	149			
Shared Lane Traffic (%)	U	U	337	U	U	17/			
ane Group Flow (vph)	Ο	Ο	350	0	0	140			
-and Group Flow (vpn) -nter Blocked Intersection	No	No	No	No	No	No			
and Alianment	L oft	Right	L off	Pight	Loft	Loft			
Jodian Width(ft)		Night	0	Nyn	Len				
ink Offsot(ft)	0		0			0			
Crosswalk Width(ft)	16		16			16			
Two way Loft Turn Lano	10		10			10			
Hoadway Eactor	1 00	1 00	1 00	1 00	1 00	1 00			
Teauway Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Turn Typo	15	9	NIA	9	15	NIΛ			
Protoctod Dhasas			2			INA 6	0		
Protected Phases			Z			0	0		
Ainimum Split (c)			2.0			2.0	2 5		
(5) Total Split (5)			3.U 2.O			3.U 2.0	3.0 2.5		
Total Split (S)			3.0			3.U 16 00/	5.5		
Javimum Croon (c)			40.Z%			40.2%	0470 1 0		
Vallow Time (s)			1.0			1.0	1.0		
			2.0			2.0	2.0		
ast Time Adjust (s)			0.0			0.0	0.5		
Lust Time Aujust (S)			0.0			0.0			
			2.0			2.0			
_eau/Lay									
Leau-Lay Optimize?			1.0			1.0			
ALL EIICI GIEEN (S)			1.0			1.0			
Actualeu y/C Kallo			0.15			0.15			
//L Kallo			1.20			0.51			
Jonitol Delay			105.0			6.8			
Lueue Delay			0.0			0.0			
lotal Delay			165.6			6.8			
_US			+			A			
Approach Delay			165.6			6.8			
Approach LOS			F			А			
	-	•	1	1	1	Ŧ			
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8		
Stops (vph)			157			7			
Fuel Used(gal)			5			0			
CO Emissions (g/hr)			366			12			
NOx Emissions (g/hr)			71			2			
VOC Emissions (g/hr)			85			3			
Dilemma Vehicles (#)			0			0			
Queue Length 50th (ft)			0			0			
Queue Length 95th (ft)			0			m0			
Internal Link Dist (ft)	148		181			1			
Turn Bay Length (ft)									
Base Capacity (vph)			281			292			
Starvation Cap Reductn			0			0			
Spillback Cap Reductn			0			0			
Storage Cap Reductn			0			0			
Reduced v/c Ratio			1.28			0.51			
Intersection Summary									
Area Type: (	Other								
Cycle Length: 6.5									
Actuated Cycle Length: 6.5									
Offset: 0 (0%), Referenced to	o phase 2:	NBT and	6:SBT, S	tart of Gre	een				
Natural Cycle: 40									
Control Type: Pretimed									
Maximum v/c Ratio: 1.28									
Intersection Signal Delay: 11	9.0			In	tersectior	n LOS: F			
Intersection Capacity Utilizat	ion 10.7%			IC	U Level o	of Service A	۱.		
Analysis Period (min) 15									
m Volume for 95th percent	ile queue i	s metereo	l by upstr	eam sign	ial.				



	-	$\mathbf{r}$	-	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				•		1
Volume (vph)	0	0	0	4	0	159
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	0	0	1900	0	1644
Flt Permitted						
Satd. Flow (perm)	0	0	0	1900	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	135			111	668	
Travel Time (s)	9.2			7.6	45.5	
Confl. Peds. (#/hr)						11
Peak Hour Factor	0.92	0.92	0.92	0.33	0.92	0.68
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	0	12	0	234
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	12	0	234
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.4%			IC	U Level of	of Service

Analysis Period (min) 15

	5	*	<b>\</b>	$\mathbf{x}$	×	4	
Lane Group	WBI	WBR	SEL	SET	NWT	NWR	ø8
Lane Configurations			JLL		1.000 T		
Volume (vnh)	0	0	1/0	<b>th</b> 1 102	126	100	
Ideal Flow (vph)	1000	1000	147	175	1000	1000	
Lano I til Eactor	1,00	1,00	0.05	0.05	1 00	1 00	
Dod Riko Factor	1.00	1.00	0.90	0.90	1.00	1.00	
Frt					0 0 2 /		
Fit Protoctod				0 072	0.724		
Satd Flow (prot)	0	0	0	3500	1726	٥	
Elt Dormitted	0	0	0	0.055	1720	0	
Satd Elow (porm)	٥	٥	٥	2110	1726	0	
Pight Turn on Pod	0	Vos	0	5440	1720	Vos	
Satd Flow (RTOD)		162			2/0	162	
Link Speed (mph)	10			10	247 10		
Link Speed (IIIpII)	10			1U 010	205		
Travel Time (c)	107			21Z	370 24 0		
Confl Dode (#/br)	IU. /	11		14.5	20.9		
Conill. Peus. (#/III) Deak Hour Factor	0 40	41	0.45	0.00	0.71	0.40	
Peak Hour Factor	0.42	0.35	0.45	0.82	U./I	0.49	
neavy venicles (%)	0%	0%	0%	U%	4%	0%	
Auj. FIOW (VPII) Sharad Lana Traffia (0()	U	U	331	235	192	249	
Shared Lane Trailic (%)	0	0	0	F//	4 4 1	0	
Lane Group Flow (Vpn)	U	U	U	566	441	U	
Enter Blocked Intersection	INO	N0	INO	INO	INO	N0	
	Lett	Right	Lett	Left	Lett	Right	
	0			0	0		
	0			0	0		
Crosswalk width(tt)	16			16	16		
Two way Lett Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
i urning Speed (mph)	15	9	15		81.4	9	
Turn Type			Perm	NA	NA		0
Protected Phases				6	2		8
Permitted Phases			6	0.6	0.0		0.5
Minimum Split (s)			3.0	3.0	3.0		3.5
Total Split (s)			3.0	3.0	3.0		3.5
I otal Split (%)			46.2%	46.2%	46.2%		54%
Maximum Green (s)			1.0	1.0	1.0		1.0
Yellow Time (s)			2.0	2.0	2.0		2.0
All-Red Time (s)			0.0	0.0	0.0		0.5
Lost Time Adjust (s)				0.0	0.0		
Total Lost Time (s)				2.0	2.0		
Lead/Lag							
Lead-Lag Optimize?							
Act Effct Green (s)				1.0	1.0		
Actuated g/C Ratio				0.15	0.15		
v/c Ratio				1.13dl	0.93		
Control Delay				70.7	35.2		
Queue Delay				0.0	0.0		
Total Delay				70.7	35.2		
LOS				E	D		

	5	*	$\searrow$	X	×	4	
Lane Group	WBL	WBR	SEL	SET	NWT	NWR	ø8
Approach Delay				70.7	35.2		
Approach LOS				Е	D		
Stops (vph)				172	59		
Fuel Used(gal)				6	3		
CO Emissions (g/hr)				428	214		
NOx Emissions (g/hr)				83	42		
VOC Emissions (g/hr)				99	50		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	77			132	315		
Turn Bay Length (ft)							
Base Capacity (vph)				530	476		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				1.07	0.93		
Intersection Summary							
Area Type: O	)ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:1	VWT and	6:SETL,	Start of C	Green		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 1.07							
Intersection Signal Delay: 55.	.2			In	tersectior	n LOS: E	
Intersection Capacity Utilization	on 45.1%			IC	U Level o	of Service	А
Analysis Period (min) 15							
dl Defacto Left Lane. Reco	de with 1 t	though la	ne as a le	eft lane.			

Splits and Phases: 14:

ø2 (R)		
3 s		
96 (R)	● <sub>ø8</sub>	
3 s	3.5 s	

# $\mathbf{A} = \mathbf{A} \mathbf{A} \mathbf{A} \mathbf{A}$

		-	•			•
Lane Group	EBL	EBR	SBL	SBR	NWL	NWR
Lane Configurations		1	ሻ	1	ሻ	
Volume (vph)	0	299	47	147	136	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865		0.850		
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	1615	1736	0
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	1615	1736	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	223		93		66	
Travel Time (s)	15.2		6.3		4.5	
Confl. Peds. (#/hr)			58			
Peak Hour Factor	0.92	0.61	0.56	0.42	0.71	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	0	490	84	350	192	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	490	84	350	192	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0	Ū	12	Ū	12	Ŭ
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Stop		Free	
Interception Summers						
Intersection Summary	Oller					
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 21.8% Analysis Period (min) 15



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	4	$\mathbf{x}$	×	₹	6	*
Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		<del>ب</del> ا	el el		Y	
Volume (vph)	14	133	104	8	9	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.986		0.941	
Flt Protected		0.995			0.973	
Satd. Flow (prot)	0	1890	1809	0	1740	0
Flt Permitted		0.995			0.973	
Satd. Flow (perm)	0	1890	1809	0	1740	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		395	105		180	
Travel Time (s)		26.9	7.2		12.3	
Confl. Peds. (#/hr)	82				118	118
Peak Hour Factor	0.50	0.51	0.65	0.42	0.25	0.46
Heavy Vehicles (%)	0%	0%	4%	0%	0%	0%
Adj. Flow (vph)	28	261	160	19	36	28
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	289	179	0	64	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: 0	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 34.3% Analysis Period (min) 15

## ۰. ٭ 5 ~ ← EBL EBT WBT SBL Lane Group WBR SBR ø2 **↑**↑ Lane Configurations Volume (vph) 0 0 0 0 116 0 Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 Lane Util. Factor 0.95 1.00 1.00 1.00 1.00 1.00 Frt Flt Protected Satd. Flow (prot) 3610 0 0 0 0 0 Flt Permitted Satd. Flow (perm) 0 0 0 0 3610 0 Right Turn on Red Yes Yes Yes Yes Satd. Flow (RTOR) Link Speed (mph) 10 10 10 Link Distance (ft) 157 118 112 Travel Time (s) 10.7 8.0 7.6 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 Heavy Vehicles (%) 0% 0% 0% 0% 0% 0% Adj. Flow (vph) 126 0 0 0 0 0 Shared Lane Traffic (%) 0 Lane Group Flow (vph) 0 126 0 0 0 Enter Blocked Intersection No No No No No No Lane Alignment Left Left Left Right Left Right Median Width(ft) 0 0 0 Link Offset(ft) 0 0 0 Crosswalk Width(ft) 16 16 16 Two way Left Turn Lane Headway Factor 1.00 1.00 1.00 1.00 1.00 1.00 Turning Speed (mph) 15 9 15 9 NA Turn Type Protected Phases 2 4 Permitted Phases Minimum Split (s) 3.0 3.5 Total Split (s) 3.0 3.5 Total Split (%) 46.2% 54% Maximum Green (s) 1.0 1.0 Yellow Time (s) 2.0 2.0 All-Red Time (s) 0.0 0.5 Lost Time Adjust (s) 0.0 Total Lost Time (s) 2.0 Lead/Lag Lead-Lag Optimize? Act Effct Green (s) 1.0 Actuated g/C Ratio 0.15 v/c Ratio 0.23 Control Delay 3.6 Queue Delay 0.0 Total Delay 3.6 LOS А Approach Delay 3.6 Approach LOS А

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	ø2		
Stops (vph)		12							
Fuel Used(gal)		0							
CO Emissions (g/hr)		21							
NOx Emissions (g/hr)		4							
VOC Emissions (g/hr)		5							
Dilemma Vehicles (#)		0							
Queue Length 50th (ft)		0							
Queue Length 95th (ft)		m0							
Internal Link Dist (ft)		77	38		32				
Turn Bay Length (ft)									
Base Capacity (vph)		555							
Starvation Cap Reductn		0							
Spillback Cap Reductn		0							
Storage Cap Reductn		0							
Reduced v/c Ratio		0.23							
Intersection Summary									
Area Type: O	Other								
Cycle Length: 6.5									
Actuated Cycle Length: 6.5									
Offset: 0 (0%), Referenced to	phase 2:H	old and	6:, Start o	of Green					
Natural Cycle: 40									
Control Type: Pretimed									
Maximum v/c Ratio: 0.23									
Intersection Signal Delay: 3.6	þ			Int	ersection	LOS: A			
Intersection Capacity Utilization	on 6.7%			IC	U Level of	f Service	А		
Analysis Period (min) 15									
m Volume for 95th percenti	le queue is	metered	l by upstr	eam signa	al.				
Splits and Phases: 2:									

, ● <sub>ø2 (R)</sub>		<b>→</b> ø4	
3.5 s		3 s	

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Lane Group	WBL	WBR	NBL	NBR	SEL	SER		
Lane Configurations	Y		Y		Y			
Volume (vph)	38	65	47	23	94	48		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Ped Bike Factor								
Frt	0.922		0.965		0.954			
Flt Protected	0.979		0.964		0.968			
Satd. Flow (prot)	1715	0	1693	0	1755	0		
Flt Permitted	0.979		0.964		0.968			
Satd. Flow (perm)	1715	0	1693	0	1755	0		
Link Speed (mph)	10		10		10			
Link Distance (ft)	408		65		105			
Travel Time (s)	27.8		4.4		7.2			
Confl. Peds. (#/hr)			47			82		
Confl. Bikes (#/hr)				47				
Peak Hour Factor	0.53	0.65	0.38	0.52	0.52	0.51		
Heavy Vehicles (%)	0%	0%	6%	0%	0%	0%		
Adj. Flow (vph)	72	100	124	44	181	94		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	172	0	168	0	275	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Right	Left	Right		
Median Width(ft)	12		12		12			
Link Offset(ft)	0		0		0			
Crosswalk Width(ft)	16		16		16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15	9	15	9		
Sign Control	Free		Stop		Free			
Intersection Summary								
Area Type:	Other							
Control Type: Unsignalized								
Intersection Capacity Utilization	tion 33.2%			IC	CU Level o	of Service A	А	
Analysis Period (min) 15								

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	ø8	
Lane Configurations			*			*		
Volume (vnh)	0	0	70	0	0	86		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
ane Util Factor	1 00	1 00	1 00	1 00	1 00	1 00		
Frt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	1827	0	0	1900		
-It Permitted	U	0	1027	U	0	1700		
Satd Flow (nerm)	0	0	1827	0	0	1900		
Right Turn on Red	U	Ves	1027	Ves	U	1700		
Satd Flow (RTOR)		105		105				
ink Snood (mnh)	10		10			10		
ink Distance (ff)	220		261			65		
Fravel Time (s)	15.5		17 Q			1.1		
Peak Hour Factor	0.02	0 00	0 00	0 02	0 02	4.4 0 0 0		
Lagy Vahiclas (%)	0.72	0.72	10/	0.72	0.72	0.72		
Adi Flow (upb)	070	0%	470	0%	0%	0%		
Sharod Lano Traffic (0/)	U	U	70	U	U	70		
and Croup Flow (upb)	0	0	74	0	0	02		
Latie Group Flow (Vpfi)	U	U	/0	U	U	93 No		
ana Alianmant	INO	N0 Dialet	INO	N0 Diaht	INO	INO Loft		
	Leit	Right	Leit	Right	Leit	Leit		
	0		0			0		
LINK Offset(ft)	0		0			0		
Crosswalk Width(ft)	16		16			16		
wo way Left Turn Lane								
leadway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
urning Speed (mph)	15	9		9	15			
furn Type			NA			NA		
Protected Phases			2			6	8	
Permitted Phases								
/linimum Split (s)			3.0			3.0	3.5	
otal Split (s)			3.0			3.0	3.5	
Total Split (%)			46.2%			46.2%	54%	
Maximum Green (s)			1.0			1.0	1.0	
Yellow Time (s)			2.0			2.0	2.0	
All-Red Time (s)			0.0			0.0	0.5	
ost Time Adjust (s)			0.0			0.0		
Fotal Lost Time (s)			2.0			2.0		
_ead/Lag								
_ead-Lag Optimize?								
Act Effct Green (s)			1.0			1.0		
Actuated g/C Ratio			0.15			0.15		
/c Ratio			0.27			0.32		
Control Delay			4.9			4.5		
Queue Delay			0.0			0.0		
Fotal Delay			4.9			4.5		
OS			A			A		
Approach Delay			4.9			4.5		
Approach LOS			A			A		

	1	*	1	1	1	Ŧ	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	Ø8
Stops (vph)			9			8	
Fuel Used(gal)			0			0	
CO Emissions (g/hr)			20			10	
NOx Emissions (g/hr)			4			2	
VOC Emissions (g/hr)			5			2	
Dilemma Vehicles (#)			0			0	
Queue Length 50th (ft)			0			0	
Queue Length 95th (ft)			0			m0	
Internal Link Dist (ft)	148		181			1	
Turn Bay Length (ft)							
Base Capacity (vph)			281			292	
Starvation Cap Reductn			0			0	
Spillback Cap Reductn			0			0	
Storage Cap Reductn			0			0	
Reduced v/c Ratio			0.27			0.32	
Intersection Summary							
Area Type: O	ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:	NBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 0.32							
Intersection Signal Delay: 4.7				Int	tersection	LOS: A	
Intersection Capacity Utilization	on 7.9%			IC	U Level o	f Service A	4
Analysis Period (min) 15							
m Volume for 95th percentil	le queue i	s meterec	l by upstr	eam sign	al.		
Culita and Dhasas. 7.							



	-	$\rightarrow$	-	-	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				•		1
Volume (vph)	0	0	0	25	0	117
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt						0.865
Flt Protected						
Satd. Flow (prot)	0	0	0	1900	0	1644
Flt Permitted						
Satd. Flow (perm)	0	0	0	1900	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	135			111	668	
Travel Time (s)	9.2			7.6	45.5	
Confl. Peds. (#/hr)						11
Peak Hour Factor	0.92	0.92	0.92	0.39	0.92	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	0	64	0	225
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	64	0	225
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 16.4%			IC	U Level of	of Service

Analysis Period (min) 15

	5	*	\ <b>\$</b>	$\mathbf{x}$	×	く		
	- \\//DI		CEI	CET.			α <sup>0</sup>	
Lane Configurations	VVDL	WDR	JEL				00	
	0	0	77	<b>M T</b>	79	20		
Idoal Elow (vphpl)	1000	1000	1000	1000	1000	1000		
Lano Litil Eactor	1 00	1,00	0.05	0.05	1,00	1,00		
Dod Piko Eactor	1.00	1.00	0.95	0.95	0.00	1.00		
				0.77	0.77			
				0.001	0.949			
Satd Elow (prot)	0	0	0	2552	1716	0		
Elt Dormittod	0	0	0	0.055	1740	0		
Satd Flow (porm)	0	0	٥	2255	17/6	٥		
Pight Turn on Red	0	Vas	U	3333	1740	Vas		
Satd Flow (PTOP)		163			56	163		
Link Sneed (mph)	10			10	10			
Link Distance (ff)	157			212	205			
Travel Time (s)	10.7			1/ 5	26.0			
Confl Peds (#/hr)	177	52	52	14.5	20.7	1		
Peak Hour Factor	0.92	0 92	0.7/	0.68	0.85	0 70		
Heavy Vehicles (%)	0.72	0.72	0.74	0.00	4%	0.70		
Adi Flow (vnh)	070	070	10/0	216	470	56		
Shared Lane Traffic (%)	U	U	104	210	72	50		
Lane Group Flow (vph)	0	0	0	320	148	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rtight	Lon	0	0	rtigrit		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane	10				10			
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type		,	Perm	NA	NA	,		
Protected Phases				6	2		8	
Permitted Phases			6	3	_			
Minimum Split (s)			3.0	3.0	3.0		3.5	
Total Split (s)			3.0	3.0	3.0		3.5	
Total Split (%)			46.2%	46.2%	46.2%		54%	
Maximum Green (s)			1.0	1.0	1.0		1.0	
Yellow Time (s)			2.0	2.0	2.0		2.0	
All-Red Time (s)			0.0	0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag				2.0	2.0			
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				0.62	0.47			
Control Delay				8.9	6.7			
Queue Delay				0.0	0.0			
Total Delay				8.9	6.7			
LOS				A	A			

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Lane Group	WBL	WBR	SEL	SET	NWT	NWR	ø8	
Approach Delay				8.9	6.7			
Approach LOS				А	А			
Stops (vph)				33	13			
Fuel Used(gal)				1	1			
CO Emissions (g/hr)				80	49			
NOx Emissions (g/hr)				16	10			
VOC Emissions (g/hr)				19	11			
Dilemma Vehicles (#)				0	0			
Queue Length 50th (ft)				0	0			
Queue Length 95th (ft)				0	0			
Internal Link Dist (ft)	77			132	315			
Turn Bay Length (ft)								
Base Capacity (vph)				516	316			
Starvation Cap Reductn				0	0			
Spillback Cap Reductn				0	0			
Storage Cap Reductn				0	0			
Reduced v/c Ratio				0.62	0.47			
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced	to phase 2:	NWT and	l 6:SETL,	Start of C	Green			
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 0.62								
Intersection Signal Delay: 8	.2			In	itersectior	n LOS: A		
Intersection Capacity Utiliza	ation 29.2%			IC	CU Level	of Service	A	
Analysis Period (min) 15								
Splits and Phases: 14:								

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Lane Configurations				**				
Volume (vph)	0	0	0	379	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	0.95	1.00	1.00		
Frt								
Flt Protected								
Satd. Flow (prot)	0	0	0	3574	0	0		
Flt Permitted								
Satd. Flow (perm)	0	0	0	3574	0	0		
Right Turn on Red	Yes	Yes	Yes			Yes		
Satd. Flow (RTOR)								
Link Speed (mph)	10			10	10			
Link Distance (ft)	76			150	44			
Travel Time (s)	5.2			10.2	3.0			
Peak Hour Factor	0.92	0.92	0.92	0.45	0.92	0.92		
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%		
Adi, Flow (vph)	0	0	0	842	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	842	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0			0	0			
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type				NA				
Protected Phases				2			4	
Permitted Phases								
Minimum Split (s)				3.0			3.5	
Total Split (s)				3.0			3.5	
Total Split (%)				46.2%			54%	
Maximum Green (s)				1.0			1.0	
Yellow Time (s)				2.0			2.0	
All-Red Time (s)				0.0			0.5	
Lost Time Adjust (s)				0.0				
Total Lost Time (s)				2.0				
Lead/Lag								
Lead-Lag Optimize?								
Act Effct Green (s)				1.0				
Actuated g/C Ratio				0.15				
v/c Ratio				1.53				
Control Delay				264.5				
Queue Delay				0.0				
Total Delay				264.5				
LOS				F				
Approach Delay				264.5				
Approach LOS				F				

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Stops (vph)				685				
Fuel Used(gal)				23				
CO Emissions (g/hr)				1584				
NOx Emissions (g/hr)				308				
VOC Emissions (g/hr)				367				
Dilemma Vehicles (#)				0				
Queue Length 50th (ft)				~15				
Queue Length 95th (ft)				0				
Internal Link Dist (ft)	1			70	1			
Turn Bay Length (ft)								
Base Capacity (vph)				549				
Starvation Cap Reductn				0				
Spillback Cap Reductn				0				
Storage Cap Reductn				0				
Reduced v/c Ratio				1.53				
Intersection Summary								
Area Type:	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced t	to phase 2:I	<b>VBT</b> and	6:, Start (	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 1.53								
Intersection Signal Delay: 20	64.5			Int	ersection	LOS: F		
Intersection Capacity Utiliza	tion 20.5%			IC	U Level o	f Service	A	
Analysis Period (min) 15								
<ul> <li>Volume exceeds capacity</li> </ul>	ty, queue is	theoretic	ally infini	te.				
Queue shown is maximu	m after two	cycles.						
Splits and Phases: 2:								
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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	60	205	312	41	8	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.983		0.973	
Flt Protected		0.988			0.962	
Satd. Flow (prot)	0	1877	1820	0	1778	0
Flt Permitted		0.988			0.962	
Satd. Flow (perm)	0	1877	1820	0	1778	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		379	107		88	
Travel Time (s)		25.8	7.3		6.0	
Confl. Peds. (#/hr)	80			2	51	
Peak Hour Factor	0.45	0.51	0.67	0.62	0.25	0.50
Heavy Vehicles (%)	0%	0%	3%	0%	0%	0%
Adj. Flow (vph)	133	402	466	66	32	8
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	535	532	0	40	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: (	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 46.4% Analysis Period (min) 15

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	4Î			
Volume (vph)	213	215	150	166	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.917			
Flt Protected		0.969				
Satd. Flow (prot)	0	1841	1700	0	0	0
Flt Permitted		0.969				
Satd. Flow (perm)	0	1841	1700	0	0	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		263	379		267	
Travel Time (s)		17.9	25.8		18.2	
Confl. Peds. (#/hr)	41			95	95	
Peak Hour Factor	0.45	0.82	0.71	0.49	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	0%	0%
Adj. Flow (vph)	473	262	211	339	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	735	550	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Free	
Intersection Summary						
Area Type: C	Other					

Control Type: Unsignalized Intersection Capacity Utilization 50.9% Analysis Period (min) 15

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Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR
Lane Configurations					र्स	el el	
Volume (vph)	0	0	100	71	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt							
Flt Protected					0.950		
Satd. Flow (prot)	0	0	0	0	1805	1900	0
Flt Permitted					0.950		
Satd. Flow (perm)	0	0	0	0	1805	1900	0
Link Speed (mph)	10				10	10	
Link Distance (ft)	558				79	105	
Travel Time (s)	38.0				5.4	7.2	
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	0	250	178	0	0	0
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	0	428	0	0
Enter Blocked Intersection	No	No	No	No	No	No	No
Lane Alignment	Left	Right	R NA	Left	Left	Left	Right
Median Width(ft)	0				0	0	
Link Offset(ft)	0				0	0	
Crosswalk Width(ft)	16				16	16	
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	9	15			9
Sign Control	Yield				Yield	Yield	
Intersection Summary							
Area Type:	Other						
Control Type: Roundabout							
Intersection Capacity Utiliza	ation 12.8%			IC	CU Level o	of Service	A
Analysis Period (min) 15							

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	•			<del>ا</del>
Volume (vph)	0	70	101	0	88	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865				
Flt Protected						0.958
Satd. Flow (prot)	0	1644	1900	0	0	1820
Flt Permitted						0.958
Satd. Flow (perm)	0	1644	1900	0	0	1820
Link Speed (mph)	10		10			10
Link Distance (ft)	164		264			132
Travel Time (s)	11.2		18.0			9.0
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	175	253	0	220	30
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	175	252	0	0	250
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Yield		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 16.3% Analysis Period (min) 15

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Lane Group	EBL	EBR	NWL	NWR	SWL	SWR
Lane Configurations		1	۲.		۲.	1
Volume (vph)	0	409	150	0	69	257
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865				0.850
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	0	1805	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	0	1805	1583
Link Speed (mph)	10		10		10	
Link Distance (ft)	88		263		95	
Travel Time (s)	6.0		17.9		6.5	
Confl. Peds. (#/hr)		1			58	
Peak Hour Factor	0.92	0.61	0.71	0.92	0.56	0.42
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%
Adj. Flow (vph)	0	670	211	0	123	612
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	670	211	0	123	612
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	tion 28.8%			IC	CU Level	of Service

Analysis Period (min) 15

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Lane Group	WBL	WBR	SEL	SER	NEL	NER
Lane Configurations	Y		¥.		Ý	
Volume (vph)	41	200	147	66	153	31
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.889		0.950		0.985	
Flt Protected	0.991		0.970		0.957	
Satd. Flow (prot)	1674	0	1751	0	1729	0
Flt Permitted	0.991		0.970		0.957	
Satd. Flow (perm)	1674	0	1751	0	1729	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	379		107		213	
Travel Time (s)	25.8		7.3		14.5	
Confl. Peds. (#/hr)	2	2	80	80	28	28
Peak Hour Factor	0.63	0.67	0.68	0.51	0.39	0.65
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	65	299	216	129	392	48
Shared Lane Traffic (%)						
Lane Group Flow (vph)	364	0	345	0	440	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type: (	Other					

Control Type: Unsignalized Intersection Capacity Utilization 51.0% Analysis Period (min) 15

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Lane Group	FBI	FBR	NBI	NBT	SBT	SBR	ø4	
Lane Configurations		LDIX	nd L			ODIX	~ 1	
Volume (vnh)	0	0	0	171	100	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane I Itil Factor	1.00	1 00	1 00	1 00	1 00	1.00		
Frt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	0	1900	1900	0		
Elt Permitted	0	U	0	1700	1700	U		
Satd Flow (perm)	0	0	0	1900	1900	0		
Right Turn on Red	U	Yes	0	1700	1700	Yes		
Satd Flow (RTOR)		105				105		
Link Speed (mph)	30			30	30			
Link Distance (ft)	145			132	79			
Travel Time (s)	33			3.0	18			
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40		
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%		
Adi Flow (vph)	0	0	0	428	250	0		
Shared Lane Traffic (%)	U	Ū	J	120	200	U		
Lane Group Flow (vph)	0	0	0	428	250	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rtigrit	Lon	0	0	rugin		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15			9		
Turn Type		·		NA	NA			
Protected Phases				2	6		4	
Permitted Phases				-				
Minimum Split (s)				3.0	3.0		3.5	
Total Split (s)				3.0	3.0		3.5	
Total Split (%)				46.2%	46.2%		54%	
Maximum Green (s)				1.0	1.0		1.0	
Yellow Time (s)				2.0	2.0		2.0	
All-Red Time (s)				0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag								
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				1.47	0.86			
Control Delay				243.8	37.4			
Queue Delay				0.0	0.0			
Total Delay				243.8	37.4			
LOS				F	D			
Approach Delay				243.8	37.4			
Approach LOS				F	D			

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4
Stops (vph)				283	28		
Fuel Used(gal)				10	1		
CO Emissions (g/hr)				715	68		
NOx Emissions (g/hr)				139	13		
VOC Emissions (g/hr)				166	16		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	65			52	1		
Turn Bay Length (ft)							
Base Capacity (vph)				292	292		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				1.47	0.86		
Intersection Summary							
Area Type: Of	ther						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:1	VBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 1.47							
Intersection Signal Delay: 167	7.7			Int	ersection	LOS: F	
Intersection Capacity Utilization	on 12.3%			IC	U Level o	of Service	A
Analysis Period (min) 15							

Splits and Phases: 19:

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3 s	3.5 s	
∮ ø6 (R)		
3 s		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4Î			र्स		1	
Volume (vph)	44	44	197	70	0	178	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt	0.932					0.865	
Flt Protected				0.964			
Satd. Flow (prot)	1771	0	0	1832	0	1644	
Flt Permitted				0.964			
Satd. Flow (perm)	1771	0	0	1832	0	1644	
Link Speed (mph)	10			10	10		
Link Distance (ft)	164			348	551		
Travel Time (s)	11.2			23.7	37.6		
Confl. Peds. (#/hr)		11					
Peak Hour Factor	0.33	0.33	0.33	0.33	0.92	0.33	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	133	133	597	212	0	539	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	266	0	0	809	0	539	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	0		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Free		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Canacity Utiliza	tion 25.6%			IC	: LL evel (	of Service	Δ

Analysis Period (min) 15

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations			<b>†</b> †			1	
Volume (vph)	0	0	379	0	0	50	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	1.00	
Frt						0.865	
Flt Protected							
Satd. Flow (prot)	0	0	3539	0	0	1611	
Flt Permitted							
Satd. Flow (perm)	0	0	3539	0	0	1611	
Link Speed (mph)		10	10		30		
Link Distance (ft)		64	172		141		
Travel Time (s)		4.4	11.7		3.2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	412	0	0	54	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	412	0	0	54	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Free		
Intersection Summary							
Area Type: 0	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	tion 20.5%			IC	U Level	of Service	ЭA
Analysis Period (min) 15							

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			•			•
Lane Group	EBL	EBR	SBL	SBR	NWL	NWR
Lane Configurations		1	۲	1	۲	
Volume (vph)	0	189	38	88	78	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.865		0.850		
Flt Protected			0.950		0.950	
Satd. Flow (prot)	0	1644	1805	1615	1736	0
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	0	1644	1805	1615	1736	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	223		93		66	
Travel Time (s)	15.2		6.3		4.5	
Confl. Peds. (#/hr)			104			
Peak Hour Factor	0.92	0.82	0.68	0.46	0.88	0.92
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	0	230	56	191	89	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	230	56	191	89	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	0		12		12	
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Stop		Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 15.0% Analysis Period (min) 15



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Lane Configurations				44				
Volume (vph)	0	0	0	214	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util Factor	1 00	1 00	1 00	0.95	1 00	1 00		
Frt				0170				
Flt Protected								
Satd, Flow (prot)	0	0	0	3574	0	0		
Flt Permitted	Ũ	Ū	Ŭ	0071	Ū	Ŭ		
Satd, Flow (perm)	0	0	0	3574	0	0		
Right Turn on Red	Yes	Yes	Yes	0071	Ū	Yes		
Satd Flow (RTOR)								
Link Speed (mph)	10			10	10			
Link Distance (ff)	76			150	44			
Travel Time (s)	5.2			10.2	3.0			
Peak Hour Factor	0.92	0.92	0.92	0.45	0.92	0.92		
Heavy Vehicles (%)	0%	0%	0%	1%	0%	0%		
Adi, Flow (vph)	0	0	0	476	0	0		
Shared Lane Traffic (%)	Ŭ	Ű	Ű	110	Ű	Ū		
Lane Group Flow (vph)	0	0	0	476	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Right	Left	Left	Left	Right		
Median Width(ft)	0	rugitt	Eon	0	0	rugin		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane	10			10	10			
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15	9	15	1.00	1.00	9		
Turn Type	10	,		NA		,		
Protected Phases				2			4	
Permitted Phases				-			•	
Minimum Split (s)				3.0			35	
Total Split (s)				3.0			3.5	
Total Split (%)				46.2%			54%	
Maximum Green (s)				10.270			10	
Yellow Time (s)				2.0			2.0	
All-Red Time (s)				0.0			0.5	
Lost Time Adjust (s)				0.0			0.0	
Total Lost Time (s)				2.0				
Lead/Lag				2.0				
Lead-Lag Ontimize?								
Act Effct Green (s)				10				
Actuated g/C Ratio				0.15				
v/c Ratio				0.10				
Control Delay				24.9				
Oueue Delay				0.0				
Total Delay				24.9				
105				<u> </u>				
Approach Delay				24.9				
Approach LOS				C				

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4	
Stops (vph)				47				
Fuel Used(gal)				2				
CO Emissions (g/hr)				150				
NOx Emissions (g/hr)				29				
VOC Emissions (g/hr)				35				
Dilemma Vehicles (#)				0				
Queue Length 50th (ft)				0				
Queue Length 95th (ft)				0				
Internal Link Dist (ft)	1			70	1			
Turn Bay Length (ft)								
Base Capacity (vph)				549				
Starvation Cap Reductn				0				
Spillback Cap Reductn				0				
Storage Cap Reductn				0				
Reduced v/c Ratio				0.87				
Intersection Summary								
Area Type: C	Other							
Cycle Length: 6.5								
Actuated Cycle Length: 6.5								
Offset: 0 (0%), Referenced to	o phase 2:1	NBT and	6:, Start o	of Green				
Natural Cycle: 40								
Control Type: Pretimed								
Maximum v/c Ratio: 0.87								
Intersection Signal Delay: 24	.9			Int	tersection	LOS: C		
Intersection Capacity Utilizati	ion 9.2%			IC	U Level o	f Service	A	
Analysis Period (min) 15								

Splits and Phases: 2:

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्स	el el		Y	
Volume (vph)	15	146	145	22	9	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.974		0.941	
Flt Protected		0.995			0.973	
Satd. Flow (prot)	0	1890	1807	0	1740	0
Flt Permitted		0.995			0.973	
Satd. Flow (perm)	0	1890	1807	0	1740	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		379	107		88	
Travel Time (s)		25.8	7.3		6.0	
Confl. Peds. (#/hr)	82				118	
Peak Hour Factor	0.50	0.51	0.65	0.42	0.25	0.46
Heavy Vehicles (%)	0%	0%	3%	0%	0%	0%
Adj. Flow (vph)	30	286	223	52	36	28
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	316	275	0	64	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		12	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type: 0	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 30.2% Analysis Period (min) 15

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Lane Group	SEL	SET	NWT	NWR	SWL	SWR
Lane Configurations		र्च	el el			
Volume (vph)	147	161	91	67	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.936			
Flt Protected		0.978				
Satd. Flow (prot)	0	1858	1745	0	0	0
Flt Permitted		0.978				
Satd. Flow (perm)	0	1858	1745	0	0	0
Link Speed (mph)		10	10		10	
Link Distance (ft)		263	379		267	
Travel Time (s)		17.9	25.8		18.2	
Confl. Peds. (#/hr)	52			4	177	
Peak Hour Factor	0.74	0.68	0.85	0.70	0.42	0.92
Heavy Vehicles (%)	0%	0%	0%	4%	0%	0%
Adj. Flow (vph)	199	237	107	96	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	436	203	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		0	0		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		16	16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Sign Control		Free	Free		Free	
Intersection Summary						
Area Type: 0	Other					

Control Type: Unsignalized Intersection Capacity Utilization 32.9% Analysis Period (min) 15

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Lane Group	EBL	EBR	NBU	NBL	NBT	SBT	SBR	
Lane Configurations					र्स	ef 👘		
Volume (vph)	0	0	74	28	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt								
Flt Protected					0.950			
Satd. Flow (prot)	0	0	0	0	1805	1900	0	
Flt Permitted					0.950			
Satd. Flow (perm)	0	0	0	0	1805	1900	0	
Link Speed (mph)	10				10	10		
Link Distance (ft)	92				79	105		
Travel Time (s)	6.3				5.4	7.2		
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40	0.40	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	0	0	185	70	0	0	0	
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	0	0	0	255	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	No	
Lane Alignment	Left	Right	R NA	Left	Left	Left	Right	
Median Width(ft)	0				0	0		
Link Offset(ft)	0				0	0		
Crosswalk Width(ft)	16				16	16		
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	9	9	15			9	
Sign Control	Yield				Yield	Yield		
Intersection Summary								
Area Type:	Other							
Control Type: Roundabout								
Intersection Capacity Utiliza	tion 9.0%			IC	CU Level o	of Service	A	
Analysis Period (min) 15								

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		1	<b>†</b>			નુ
Volume (vph)	0	65	37	0	52	22
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.865				
Flt Protected						0.966
Satd. Flow (prot)	0	1644	1900	0	0	1835
Flt Permitted						0.966
Satd. Flow (perm)	0	1644	1900	0	0	1835
Link Speed (mph)	10		10			10
Link Distance (ft)	164		264			132
Travel Time (s)	11.2		18.0			9.0
Peak Hour Factor	0.40	0.40	0.40	0.40	0.40	0.40
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	0	163	93	0	130	55
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	162	92	0	0	185
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	0		0			0
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Sign Control	Yield		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						

Intersection Capacity Utilization 14.0% Analysis Period (min) 15
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Lane Group	EBL	EBR	NWL	NWR	SWL	SWR	
Lane Configurations		1	5		5	1	
Volume (vph)	0	256	91	0	52	154	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor							
Frt		0.865				0.850	
Flt Protected			0.950		0.950		
Satd. Flow (prot)	0	1644	1805	0	1805	1583	
Flt Permitted			0.950		0.950		
Satd. Flow (perm)	0	1644	1805	0	1805	1583	
Link Speed (mph)	10		10		10		
Link Distance (ft)	88		263		95		
Travel Time (s)	6.0		17.9		6.5		
Confl. Peds. (#/hr)					104		
Peak Hour Factor	0.92	0.82	0.88	0.92	0.68	0.46	
Heavy Vehicles (%)	0%	0%	0%	0%	0%	2%	
Adj. Flow (vph)	0	312	103	0	76	335	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	312	103	0	76	335	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Right	Left	Right	
Median Width(ft)	0		12		12		
Link Offset(ft)	0		0		0		
Crosswalk Width(ft)	16		16		16		
I wo way Left Turn Lane	1.00	1.00	1.00	1.00	1.00	1.00	
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Furning Speed (mph)	15	9	15	9	15 Char	9	
Sign Control	Free		Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalized							
Intersection Capacity Utiliza	tion 19.2%			IC	CU Level	of Service	: A

Analysis Period (min) 15

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Lane Group	WBL	WBR	SEL	SER	NEL	NER
Lane Configurations	Y		Y		Y	
Volume (vph)	51	92	102	53	75	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.906		0.953		0.975	
Flt Protected	0.985		0.968		0.961	
Satd. Flow (prot)	1696	0	1753	0	1724	0
Flt Permitted	0.985		0.968		0.961	
Satd. Flow (perm)	1696	0	1753	0	1724	0
Link Speed (mph)	10		10		10	
Link Distance (ft)	379		107		213	
Travel Time (s)	25.8		7.3		14.5	
Confl. Peds. (#/hr)		2	82		47	
Peak Hour Factor	0.53	0.42	0.52	0.51	0.38	0.52
Heavy Vehicles (%)	0%	0%	0%	0%	4%	0%
Adj. Flow (vph)	96	219	196	104	197	44
Shared Lane Traffic (%)						
Lane Group Flow (vph)	315	0	300	0	241	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Right
Median Width(ft)	12	0	12	5	12	5
Link Offset(ft)	0		0		0	
Crosswalk Width(ft)	16		16		16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	15	9	15	9
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type: (	Other					
Control Type, Unclandized						

Control Type: Unsignalized Intersection Capacity Utilization 33.4% Analysis Period (min) 15

ICU Level of Service A

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Lane Group	FBI	FBR	NBI	NBT	SBT	SBR	ø4	
Lane Configurations		2011		*	•	0.511	~ .	
Volume (vnh)	0	0	0	102	7/	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane I Itil Factor	1 00	1 00	1 00	1 00	1.00	1.00		
Frt	1.00	1.00	1.00	1.00	1.00	1.00		
Flt Protected								
Satd Flow (prot)	0	0	0	1900	1900	0		
Elt Permitted	U	U	U	1700	1700	U		
Satd Flow (perm)	0	0	0	1900	1900	0		
Right Turn on Red	U	Yes	U	1700	1700	Yes		
Satd Flow (RTOR)		105				105		
Link Speed (mph)	30			30	30			
Link Distance (ft)	1/5			132	70			
Travel Time (s)	2 2			30	1.8			
Peak Hour Factor	0.40	0 40	0.40	0.40	0.40	0.40		
Heavy Vehicles (%)	0.40	0.40	0.40	0.40	0.40	0.40		
Adi Flow (vph)	070	070	070	255	125	070		
Shared Lane Traffic (%)	U	U	0	200	105	U		
Lane Group Flow (vph)	0	0	0	255	185	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	I eft	Right	l eft	l eft	l eft	Right		
Median Width(ft)	0	Right	Lon	0	0	Right		
Link Offset(ft)	0			0	0			
Crosswalk Width(ft)	16			16	16			
Two way Left Turn Lane	10			10	10			
Headway Factor	1 00	1 00	1 00	1 00	1 00	1 00		
Turning Speed (mph)	15	9	15	1.00	1.00	9		
Turn Type	10	,	10	NA	NA	,		
Protected Phases				2	6		4	
Permitted Phases				۷	Ū			
Minimum Split (s)				3.0	3.0		3.5	
Total Split (s)				3.0	3.0		3,5	
Total Split (%)				46.2%	46.2%		54%	
Maximum Green (s)				1.0	1.0		1.0	
Yellow Time (s)				2.0	2.0		2.0	
All-Red Time (s)				0.0	0.0		0.5	
Lost Time Adjust (s)				0.0	0.0			
Total Lost Time (s)				2.0	2.0			
Lead/Lag				2.5	2.0			
Lead-Lag Optimize?								
Act Effct Green (s)				1.0	1.0			
Actuated g/C Ratio				0.15	0.15			
v/c Ratio				0.87	0.63			
Control Delav				40.1	16.3			
Queue Delay				0.0	0.0			
Total Delay				40.1	16.3			
LOS				D	В			
Approach Delav				40.1	16.3			
Approach LOS				D	В			

Shaw School 4/9/2014 Baseline

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	ø4
Stops (vph)				30	13		
Fuel Used(gal)				1	0		
CO Emissions (g/hr)				77	25		
NOx Emissions (g/hr)				15	5		
VOC Emissions (g/hr)				18	6		
Dilemma Vehicles (#)				0	0		
Queue Length 50th (ft)				0	0		
Queue Length 95th (ft)				0	0		
Internal Link Dist (ft)	65			52	1		
Turn Bay Length (ft)							
Base Capacity (vph)				292	292		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				0.87	0.63		
Intersection Summary							
Area Type: Ot	her						
Cycle Length: 6.5							
Actuated Cycle Length: 6.5							
Offset: 0 (0%), Referenced to	phase 2:I	VBT and	6:SBT, S	tart of Gre	een		
Natural Cycle: 40							
Control Type: Pretimed							
Maximum v/c Ratio: 0.87							
Intersection Signal Delay: 30.1				Int	ersection	LOS: C	
Intersection Capacity Utilizatio	n 8.7%			IC	U Level o	f Service /	Ą
Analysis Period (min) 15							

Splits and Phases: 19:

Ø2 (R)	● <sub>ø4</sub>	
3 s	3.5 s	
Ø6 (R)		
3 s		

	-	$\mathbf{r}$	1	+	1	1
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	eî.			र्स		1
Volume (vph)	26	26	117	65	0	125
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932					0.865
Flt Protected				0.967		
Satd. Flow (prot)	1771	0	0	1837	0	1644
Flt Permitted				0.967		
Satd. Flow (perm)	1771	0	0	1837	0	1644
Link Speed (mph)	10			10	10	
Link Distance (ft)	164			348	551	
Travel Time (s)	11.2			23.7	37.6	
Confl. Peds. (#/hr)		25				
Peak Hour Factor	0.33	0.33	0.33	0.39	0.92	0.33
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	79	79	355	167	0	379
Shared Lane Traffic (%)						
Lane Group Flow (vph)	158	0	0	522	0	379
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Left	Left	Right
Median Width(ft)	0			0	0	
Link Offset(ft)	0			0	0	
Crosswalk Width(ft)	16			16	16	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)		9	15		15	9
Sign Control	Free			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliza	tion 23.7%			IC	U Level	of Service

Intersection Capacity Utili Analysis Period (min) 15